

Tritax Symmetry (Hinckley) Limited

HINCKLEY NATIONAL RAIL FREIGHT INTERCHANGE

The Hinckley National Rail Freight Interchange Development Consent Order

Project reference TR050007

Environmental Statement Volume 2: Appendices

Appendix 8.1: Transport Assessment [part 10 of 20] VISSIM LMVR Base Models

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November 2022

Planning Act 2008

The Infrastructure Planning (Applications: Prescribed Forms and Procedure) Regulations 2009
Regulation 5(2)(a)

The Infrastructure Planning (Environmental Impact Assessment) Regulations 2017
Regulation 14

This document forms a part of the Environmental Statement for the Hinckley National Rail Freight Interchange project.

Tritax Symmetry (Hinckley) Limited (TSH) has applied to the Secretary of State for Transport for a Development Consent Order (DCO) for the Hinckley National Rail Freight Interchange (HNRFI).

To help inform the determination of the DCO application, TSH has undertaken an environmental impact assessment (EIA) of its proposals. EIA is a process that aims to improve the environmental design of a development proposal, and to provide the decision maker with sufficient information about the environmental effects of the project to make a decision.

The findings of an EIA are described in a written report known as an Environmental Statement (ES). An ES provides environmental information about the scheme, including a description of the development, its predicted environmental effects and the measures proposed to ameliorate any adverse effects.

Further details about the proposed Hinckley National Rail Freight Interchange are available on the project website:



The DCO application and documents relating to the examination of the proposed development can be viewed on the Planning Inspectorate's National Infrastructure Planning website:

<https://infrastructure.planninginspectorate.gov.uk/projects/east-midlands/hinckley-national-rail-freight-interchange/>



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Tritax Symmetry Ltd
Hinckley National Rail Freight Interchange
M69 Junction 1 VISSIM
Local Model Validation Report



TRANSPORT & INFRASTRUCTURE PLANNING

Tritax Symmetry Ltd
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M69 Junction 1 VISSIM
Local Model Validation Report

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1. INTRODUCTION

Instruction

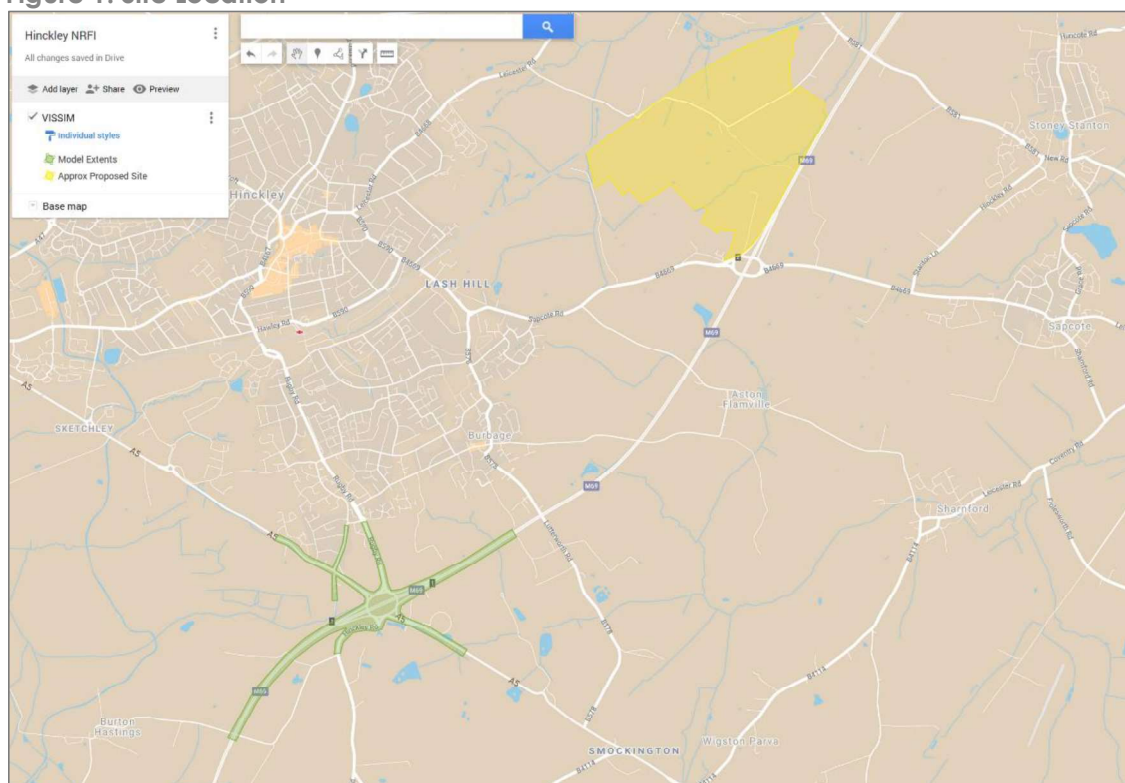
BWB Consulting has been commissioned as part a wider project scope by Tritax Symmetry Ltd to develop a series of highway models capable assessing any highway impacts resultant of the proposed Hinckley National Rail Freight Interchange (HNRFI) development. It is understood that the site will be developed serving a maximum of 850,000sqm of B8 warehousing/distribution uses, with access served directly onto M69 Junction 2.

The model purpose is to provide a robust platform on which the proposed development can be tested, allowing any impacts on the junction and surrounding highway network to be assessed.

Site Location

- 1.1 **Figure 1** below displays the indicative location of the proposed development, as well as the relative position of the highway model extents.

Figure 1: Site Location



Report Purpose

- 1.2 Due to the scale of the proposed development and the likely vehicular trips that it will generate, a comprehensive micro-simulation model of the M69 Junction 1 gyratory has been developed using PTV Group's VISSIM software.

- 1.3 The following Local Model Validation Report (LMVR) summarises the methodology used to build and test the model, as well as the results obtained to determine the suitability of the model for use in proposed option testing.
- 1.4 Following the completion of the validation process, the model will be submitted for approval to Highways England (HE) and Leicestershire County Council (LCC) as the Local Highway Authority (LHA), for review, comment, and agreement.
- 1.5 This LMVR seeks to define in detail the process and procedures followed in the development of the modelled network and the methods applied in the traffic modelling itself.

2. REPORT STRUCTURE

The report is structured as follows:

- Section 2: Base Model Development including details on the software used, the model extents alteration process, duration and any changes made to software parameters in line with best-practice recommendations;
- Section 3: Base Model Calibration including a comparison of the previous model with this cordoned model, as well as observed and modelled turning flows;
- Section 4: Model Validation including the comparison of observed and modelled journey times; and
- Section 5: Summary and Recommendations including a summary of the model development process and the overall suitability for future use.

3. BASE MODEL DEVELOPMENT

Model Specification

3.1 VISSIM Version – 20.00-14

3.2 Model Base Year – 2019

3.3 Model Time Periods:

- Weekday AM – 07:00-07:30 (warm-up), 07:30-09:30 (peak period/s), 09:30-10:00 (cool-down)
- Weekday PM – 16:00-16:30 (warm-up), 16:30-18:30 (peak period/s), 18:30-19:00 (cool-down)

3.4 Model units have been specified as:

- Metres (m);
- Kilometres (km);
- Miles per hour (mph); and
- Metres / second squared (m/s²).

3.5 Vehicle Types Used:

- Cars
- LGV
- HGV (OGV1 & OGV2)
- Coaches
- Motorcycles

3.6 Geometric calculations for base model construction were derived from OS Master mapping in combination with high resolution aerial imagery, overlaid. A check on the accuracy of the base map was undertaken against online satellite imagery and Google Street view, and where discrepancies were identified this was accounted for in network coding within VISSIM. These calculations have informed the lane width, link length and number of lane parameters within the model.

3.7 The emergency stop and lane change parameters have been used to model lane change behaviour. These were determined by reviewing the physical characteristics of specific parts of the network. The values used depend on a number of factors including positioning of signing, type of junction, general visibility and proximity of other junctions. The emergency stop distance specifies the last possible position value for a vehicle to change lanes. The emergency stop value has been left at a default value of 5m except where longer queue lengths are modelled.

- 3.8 Results have been output with a model resolution of 10-time steps per second, as a result of the requirements of the external signal control module, PC MOVA. Random seeds used were set with a starting seed of 42, with an incremental increase of 41.

Base Data – Changes from Default Driving Behaviour Parameters

- 3.9 The base year network primarily makes use of three driver behaviour profiles:
- 1 – Urban (motorized)
 - Driver behaviour profile 1 is largely left as per the PTV default settings. As a result of previous experience and reference to TfL guidelines for urban roads, two changes have been made to the **Following** rules. As a result of the more complex set-up of physical elements within combined junctions, the number of interaction objects has been increased to **8**. The number of interaction vehicles has been reduced to **4**, as this is considered more realistic in an urban setting. Look ahead and look back distances have also been updated to allow for more realistic behaviour in congested conditions.
 - 2 – Left-side rule (motorized)
 - Driver behaviour profile 2 is left as per the PTV default settings.
 - 7 – Urban (Aggressive merge)
 - Driver behaviour profile 7 is a bespoke behaviour created to allow more aggressive merging behaviour. The template used is Driver behaviour profile 1. Changes include a decrease to the Number of interaction vehicles from **4** to **2**, a reduction of average standstill distance from **2m** to **1.5m**, and the use of Co-operative lane change, rather than advanced merging. Maximum deceleration for co-operative braking has been increased to **-9.00 m/s²**.

Base Data – Changes from Default Desired Speed Profiles

- 3.10 Distribution profiles for the 20mph, 30mph, 40mph, 50mph, National Speed Limit (NSL) Single Carriageway, NSL Dual Carriageway, and NSL motorway have been taken from the latest available DfT National Speed statistics.
- 3.11 Distribution profiles have also been created for use with Reduced Speed Area controls (RSA) on corners, as well as to control saturation flow rates at signal stop lines. Reduced Speed Areas are used throughout the model in locations where a bend is of such a radius that it will always require a motorist to brake when negotiating it. It is important to understand that a Reduced Speed Area upon a bend will actually result in a vehicle decelerating on the approach to the bend, rather than upon it.

Model Assignment

- 3.12 Although the network has no route choice, the dynamic assignment module was chosen for model assignment due to the relative ease of entering traffic flows via Origin-Destination (OD) matrices for both the base development and the addition of future year growth at a later stage.
- 3.13 To provide an accurate traffic profile, traffic OD matrices have been created per vehicle type, for each 15-minute interval, in line with the collected data. It should be

noted that although the traffic has been separated into 15-minute intervals, the model has been validated hourly for each of the two busiest hours in each peak period.

- 3.14 As there is no route choice in the model, there is not judged to be any need for the process of route convergence, however the path and cost files used were run at least 20 times in order to ensure stability.

4. BASE MODEL CALIBRATION

This section summarises the calibration process undertaken and identifies sources of traffic flow data used to check and refine the flow profiles within the VISSIM model.

TRAFFIC FLOW SOURCES

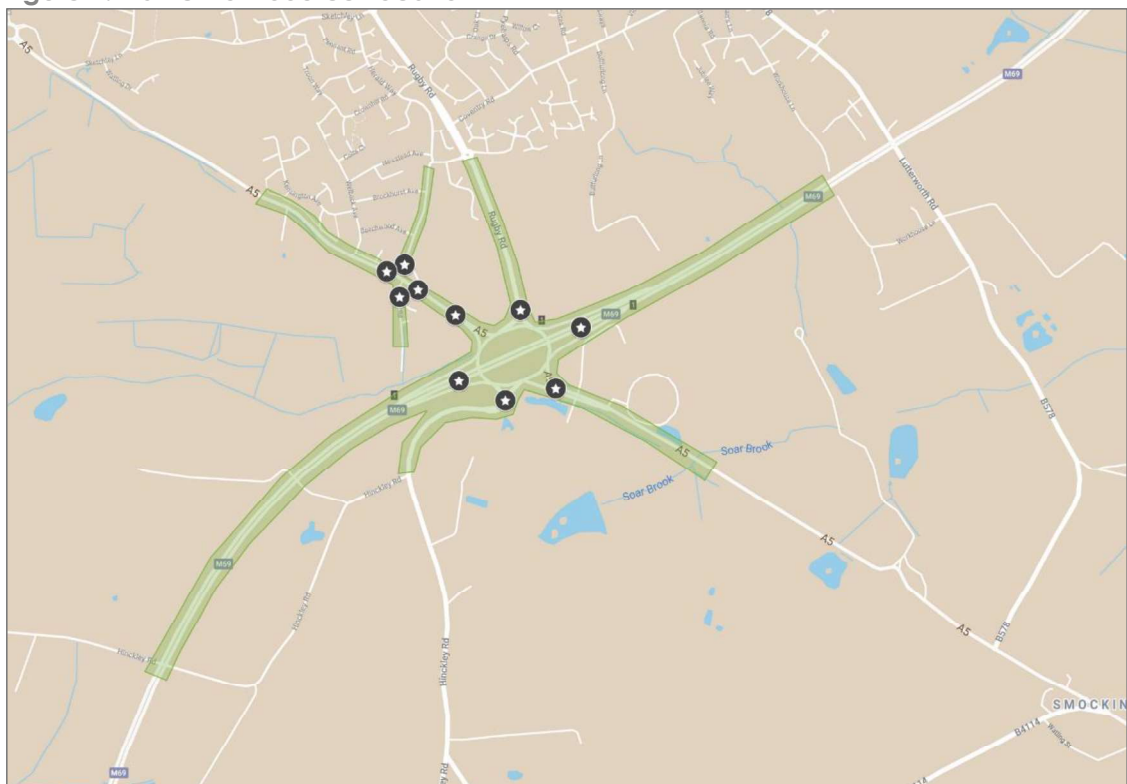
4.1 Manual Classified Count (MCC) surveys were undertaken on 10th April 2019 at the following locations:

- M69 Junction 1
- A5/Wolvey Road Junction

4.2 Link counts (10th April 2019) have been acquired from the WebTRIS database at the following site locations:

- M69 mainline flow (3540) – northbound, north of M69 Junction 1
- M69 mainline flow (4566) – southbound, north of M69 Junction 1
- M69 mainline flow (5024) – northbound, south of M69 Junction 1
- M69 mainline flow (4189) – southbound, south of M69 Junction 1

Figure 2: Traffic Flow Source Location



TRAFFIC FLOW CALIBRATION

- 4.3 The process of flow calibration has involved multiple iterations of minor adjustments to priority control at key locations and on key routes. The calculated GEH statistic for the observed and modelled flows was considered for each of the junction turning counts in accordance with the criteria stated in TAG Unit 3.1. To consider day to day variation in driver behaviour, the models were run, and results averaged over twenty random seeds, as per the original model specification. **Tables 1-6** summarise the flow calibration results.
- 4.4 For transparency, completeness and robustness, these results also include a comparison against the TfL criteria for key links, using a GEH value of 3 or under. It has now been possible to achieve the ideal minimum 85% count, demonstrating that a strong flow calibration result has been achieved. A full breakdown of model calibration results can be found in **Appendix A**.

Table 1: AM Flow Calibration – 0730-0830hrs

AM Peak (07:30-08:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 2: AM Flow Calibration – 0830-0930hrs

AM Peak (08:30-09:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 3: AM Flow Calibration – 0730-0930hrs

AM Peak (07:30-09:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 4: PM Flow Calibration – 1630-1730hrs

PM Peak (16:30-17:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 5: PM Flow Calibration – 1730-1830hrs

PM Peak (17:30-18:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 6: PM Flow Calibration – 1630-1830hrs

PM Peak (16:30-18:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

TRAFFIC SIGNAL CALIBRATION

- 4.5 Traffic signals have been modelled using the PC MOVA emulation module. MOVA config files had been provided by HE, however it was identified that there were some compatibility issues as a result of differences between what the MOVA kernel can do, and the functions available to PC MOVA.
- 4.6 A new PC MOVA config was therefore created in order to allow full co-ordination between the three separate controllers, as is found on-site.

5. BASE MODEL VALIDATION

This section summarises the goodness of fit between modelled and observed outputs, independently collected.

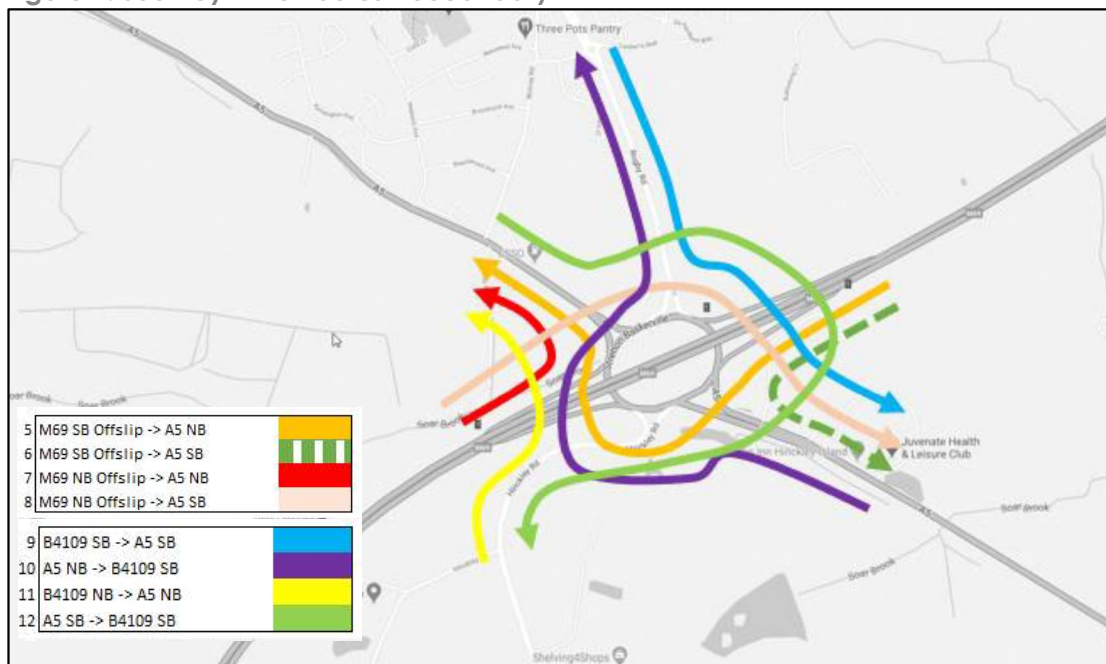
VEHICLE JOURNEY TIME VALIDATION

- 5.1 The journey time validation has been carried out using TomTom data collected for the network. This was chosen as it provides a high sample rate dataset which improves the overall robustness of the validation comparison.
- 5.2 The data is provided in small link sections, so these were combined into more reasonable lengths from junction to junction in the network, which assisted the calibration of the model. For the purpose of providing journey time validation, multiple sections have been combined into longer journey routes, covering all major movements at key locations.
- 5.3 A total of 12 journey time routes have been prepared for the purpose of model validation. **Figure 3** shows the location of four, primary through routes. **Figure 4** shows the location of eight secondary turning routes.

Figure 3: Journey Time Routes - Primary



Figure 4: Journey Time Routes - Secondary



JOURNEY TIME DATA

5.4 **Tables 7 and 8** below shows the overall summary for all journey time routes and sections for the network. See **Appendix B** for more detailed tables for each route.

Table 7: AM Journey Time Validation

Whole Routes	AM Peak		
	07:30-08:30	08:30-09:30	07:30-09:30
Criteria			
85% of measures within 15%	83%	100%	92%
85% of measures within 60 seconds	100%	100%	100%

Table 8: PM Journey Time Validation

Whole Routes	PM Peak		
	16:30-17:30	17:30-18:30	16:30-18:30
Criteria			
85% of measures within 15%	100%	83%	100%
85% of measures within 60 seconds	100%	100%	100%

5.5 In accordance with TAG Unit 3.1 criteria, which recommends that the difference between observed and modelled journey times should be within 15% (or 1 minute if higher) for at least 85% of the routes evaluated (although that criteria is ideally designed for route sections over 3km and under 15km in length) it can be seen from **Tables 7 and 8** that all routes meet one or both criteria in the AM and PM peak models.

5.6 In the AM peak, the 0830-0930hrs and 0730-0930hrs time periods both meet the TAG criteria, with over 85% of the routes being within 15% and 60s. In the 0730-0830hrs time period, there are two routes which fall outside of the 15% difference (one having a 16% difference) and 12/12 routes are within 60s. Given how close the non-validating route is to 15% difference, the model is still considered representative of on-street conditions.

- 5.7 In the PM peak, 12/12 routes are within 15% and 60s for the 1630-1730hrs and 1630-1830hrs time periods. In the 1730-1830hrs time period, there are two routes which fall outside of the 15% difference (the two routes having an 18% and 20% difference) and 12/12 routes are within 60s. Given how close the non-validating routes are to 15% difference, the model is still considered representative of on-street conditions.
- 5.8 Overall, this is a robust validation result, indicative of a good likeness between modelled performance and on-street conditions.

6. SUMMARY AND CONCLUSIONS

SUMMARY

- 6.1 This LMVR documents the development of the base model and demonstrates that it is an acceptable representation of the highway network within the study area and is fit for the purpose of developing traffic forecasts to assess the impact of development proposal scheme on the M69 Junction 1 gyratory.
- 6.2 The purpose of model calibration is to ensure that the model assignments are appropriate. The main emphasis of the calibration is to ensure that the model accurately reflects existing conditions during the modelling period with regard to:
- Traffic patterns;
 - Key junctions; and
 - Traffic volumes and routing.
- 6.3 In regard to the traffic turning and flow counts at the surveyed sites the model exceeds the 85% criteria set by TAG Unit 3.1.
- 6.4 The model has been validated to observed journey times within the extents of the network. The data has been provided as 15-minute intervals and an average one-hour journey time for each of the peak network periods.
- 6.5 The journey time comparisons show consistency between the modelled and observed journey time profiles across the majority of the journey time routes. In the AM and PM peaks, there are two routes which fall outside of the 15% range (0730-08hrs and 1730-1830hrs). However, all of the routes are within 60s and as such, the journey times are considered representative.
- 6.6 Given that the traffic flows and journey times compare well with on-site conditions, it should be considered a successful calibration and validation exercise.

CONCLUSION

- 6.7 It is understood that as these conditions are met and are of a sufficient quality to represent real world conditions the M69 Junction 1 gyratory VISSIM model is considered robust and acceptable for testing of the proposed development.

APPENDICES

APPENDIX 1: Flow Calibration

PM Peak (16:30-18:30) Summary - ALL

Total number of counts considered	42
VBSIM model counts with GEH < 3	42
% of VBSIM counts with GEH < 3	100.00%
VBSIM model counts with GEH < 5	42
% of VBSIM counts with GEH < 5	100.00%
VBSIM model counts with GEH < 10	42
% of VBSIM counts with GEH < 10	100.00%
VBSIM model counts meeting WvRTAG Uln 3.1 criteria	42
% of VBSIM counts meeting WvRTAG Uln 3.1 flow criteria	100.00%

Junction	Junction / Movement		Vehicle Flow				Difference		GEH Criteria Met			Flow Criteria Met	
	Approach	Exit/Movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	Flow	<700	>2700	
M69 Junction 1	B4109 Rugby Road	M69 Eastbound	117	117	0	0%	N	0.00	*	*			
		A5 Southbound	235	236	1	0%	N	0.07	*	*			
		B4109 Hinckley Road	259	259	0	0%	N	0.00	*	*			
		M69 Westbound	387	363	-24	-6%	N	1.24	*	*			
		A5 Northbound	32	31	-1	-3%	N	0.18	*	*			
	M69 Westbound	A5 Southbound	64	65	1	2%	N	0.13	*	*			
		B4109 Hinckley Road	377	380	3	1%	N	0.14	*	*			
		M69 Westbound	3152	3155	3	0%	N	0.05	*	*	*	*	
		A5 Northbound	456	454	-2	-0%	N	0.10	*	*			
		B4109 Rugby Road	179	178	-1	-1%	N	0.08	*	*			
		B4109 Hinckley Road	15	15	0	0%	N	0.00	*	*			
		M69 Westbound	265	267	2	1%	N	0.13	*	*			
		A5 Northbound	787	766	-21	-3%	N	0.78	*	*			
		B4109 Rugby Road	465	462	-3	-1%	N	0.15	*	*			
		M69 Eastbound	44	45	1	2%	N	0.15	*	*			
		M69 Westbound	27	27	0	0%	N	0.00	*	*			
		A5 Northbound	192	189	-3	-2%	N	0.07	*	*			
		B4109 Rugby Road	373	376	3	1%	N	0.16	*	*			
		M69 Eastbound	181	185	4	2%	N	0.30	*	*			
		B4109 Hinckley Road	A5 Southbound	14	14	0	0%	N	0.00	*	*		
	A5 Northbound		747	743	-4	-1%	N	0.11	*	*	*	*	
	M69 Eastbound		1258	1262	4	0%	N	0.11	*	*	*	*	
	M69 Eastbound		4516	4530	14	0%	N	0.21	*	*	*	*	
	A5 Southbound		681	682	1	0%	N	0.04	*	*			
	B4109 Hinckley Road		34	34	0	0%	N	0.00	*	*			
	B4109 Rugby Road		32	32	0	0%	N	0.00	*	*			
	M69 Eastbound		456	458	2	0%	N	0.09	*	*			
	A5 Southbound		806	805	-1	-0%	N	0.03	*	*	*	*	
	B4109 Hinckley Road		86	76	-10	-12%	N	1.11	*	*			
	M69 Eastbound	M69 Westbound	473	470	-3	-1%	N	0.14	*	*			
		A5 Watling Street Eastbound	52	49	-3	-6%	Y	0.42	*	*			
		Wolvey Road Southbound	1	1	0	0%	N	0.00	*	*			
		A5 Watling Street Westbound	41	41	0	0%	Y	0.00	*	*			
		Wolvey Road Southbound	1	0	-1	-100%	Y	1.41	*	*			
		A5 Watling Street Westbound	2030	2095	65	3%	Y	1.43	*	*	*	*	
		Wolvey Road Northbound	93	91	-2	-2%	Y	0.21	*	*			
		A5 Watling Street Westbound	1	1	0	0%	N	0.00	*	*			
		Wolvey Road Northbound	1	1	0	0%	N	0.00	*	*			
		A5 Watling Street Eastbound	1	0	-1	-100%	N	1.41	*	*			
		Wolvey Road Northbound	300	297	-3	-1%	Y	0.17	*	*	*	*	
Wolvey Road Northbound		1867	1862	-5	-0%	Y	0.37	*	*	*	*		
A5 Watling Street Eastbound		0	0	0	0%	Y	0.00	*	*	*	*		
A5 Watling Street Westbound		0	0	0	0%	Y	0.00	*	*	*	*		

PM Peak (16:30-18:30) Summary - LIGHTS

Total number of counts considered	42
VBSIM model counts with GEH < 3	42
% of VBSIM counts with GEH < 3	100.00%
VBSIM model counts with GEH < 5	42
% of VBSIM counts with GEH < 5	100.00%
VBSIM model counts with GEH < 10	42
% of VBSIM counts with GEH < 10	100.00%
VBSIM model counts meeting WvRTAG Uln 3.1 criteria	42
% of VBSIM counts meeting WvRTAG Uln 3.1 flow criteria	100.00%

Junction	Junction / Movement		Vehicle Flow				Difference		GEH Criteria Met			Flow Criteria Met	
	Approach	Exit/Movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	Flow	<700	>2700	
M69 Junction 1	B4109 Rugby Road	M69 Eastbound	114	114	0	0%	N	0.00	*	*			
		A5 Southbound	230	232	2	1%	N	0.13	*	*			
		B4109 Hinckley Road	236	235	-1	-0%	N	0.07	*	*			
		M69 Westbound	384	360	-24	-6%	N	1.24	*	*			
		A5 Northbound	31	30	-1	-3%	N	0.18	*	*			
	M69 Westbound	A5 Southbound	57	58	1	2%	N	0.13	*	*			
		B4109 Hinckley Road	369	372	3	1%	N	0.16	*	*			
		M69 Westbound	2840	2842	2	0%	N	0.04	*	*	*	*	
		A5 Northbound	424	423	-1	-0%	N	0.05	*	*			
		B4109 Rugby Road	177	177	0	0%	N	0.00	*	*			
		B4109 Hinckley Road	14	14	0	0%	N	0.00	*	*			
		M69 Westbound	242	243	1	0%	N	0.06	*	*			
		A5 Northbound	676	661	-15	-2%	N	0.70	*	*			
		B4109 Rugby Road	400	397	-3	-1%	N	0.15	*	*			
		M69 Eastbound	43	43	0	0%	N	0.00	*	*			
		M69 Westbound	27	27	0	0%	N	0.00	*	*			
		A5 Northbound	186	187	1	1%	N	0.07	*	*			
		B4109 Hinckley Road	B4109 Rugby Road	371	374	3	1%	N	0.16	*	*		
		M69 Eastbound	179	182	3	2%	N	0.12	*	*			
		A5 Southbound	13	13	0	0%	N	0.00	*	*			
	A5 Northbound	681	678	-3	-0%	N	0.12	*	*				
	M69 Eastbound	B4109 Rugby Road	1253	1259	6	0%	N	0.17	*	*	*	*	
		M69 Eastbound	4018	4041	23	0%	N	0.20	*	*	*	*	
		A5 Southbound	647	648	1	0%	N	0.04	*	*			
		B4109 Hinckley Road	33	33	0	0%	N	0.00	*	*			
		B4109 Rugby Road	32	32	0	0%	N	0.00	*	*			
		M69 Eastbound	424	427	3	1%	N	0.15	*	*			
		A5 Southbound	815	815	0	0%	N	0.00	*	*	*	*	
		B4109 Hinckley Road	84	74	-10	-12%	N	1.13	*	*			
		M69 Westbound	443	440	-3	-1%	N	0.14	*	*			
		A5 Watling Street Eastbound	50	47	-3	-6%	Y	0.43	*	*			
	M69 Eastbound	Wolvey Road Southbound	1	1	0	0%	N	0.00	*	*			
		A5 Watling Street Westbound	40	43	3	1%	Y	0.16	*	*			
		Wolvey Road Southbound	1	0	-1	-100%	Y	1.41	*	*			
		A5 Watling Street Westbound	1816	1887	71	4%	Y	1.65	*	*	*	*	
		Wolvey Road Northbound	91	91	0	0%	N	0.00	*	*			
		A5 Watling Street Westbound	1	1	0	0%	N	0.00	*	*			
		Wolvey Road Northbound	0	0	0	0%	N	0.00	*	*			
		A5 Watling Street Eastbound	1	0	-1	-100%	N	1.41	*	*			
		Wolvey Road Northbound	297	294	-3	-1%	Y	0.17	*	*	*	*	
A5 Watling Street Eastbound		1717	1732	15	1%	Y	0.36	*	*	*	*		
A5 Watling Street Westbound		0	0	0	0%	Y	0.00	*	*	*	*		

PM Peak (16:30-18:30) Summary - HEAVIES

Total number of counts considered	42
VBSIM model counts with GEH < 3	42
% of VBSIM counts with GEH < 3	100.00%
VBSIM model counts with GEH < 5	42
% of VBSIM counts with GEH < 5	100.00%
VBSIM model counts with GEH < 10	42
% of VBSIM counts with GEH < 10	100.00%
VBSIM model counts meeting WvRTAG Uln 3.1 criteria	42
% of VBSIM counts meeting WvRTAG Uln 3.1 flow criteria	100.00%

Junction	Junction / Movement		Vehicle Flow				Difference		GEH Criteria Met			Flow Criteria Met	
	Approach	Exit/Movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	Flow	<700	>2700	
M69 Junction 1	B4109 Rugby Road	M69 Eastbound	3	3	0	0%	Y	0.00	*	*			
		A5 Southbound	5	4	-1	-20%	Y	0.47	*	*			
		B4109 Hinckley Road	3	4	1	33%	Y	0.53	*	*			
		M69 Westbound	3	3	0	0%	Y	0.00	*	*			
		A5 Northbound	1	1	0	0%	Y	0.00	*	*			
	M69 Westbound	A5 Southbound	7	7	0	0%	Y	0.00	*	*			
		B4109 Hinckley Road	8	8	0	0%	Y	0.00	*	*			
		M69 Westbound	312	313	1	0%	Y	0.06	*	*			
		A5 Northbound	32	31	-1	-3%	Y	0.18	*	*			
		B4109 Rugby Road	2	1	-1	-50%	Y	0.82	*	*			
		B4109 Hinckley Road	1	1	0	0%	Y	0.00	*	*			
		M69 Westbound	23	24	1	4%	Y	0.21	*	*			
		A5 Northbound	108	105	-3	-3%	Y	0.29	*	*			
		B4109 Rugby Road	5	5	0	0%	Y	0.00	*	*			
		M69 Eastbound	1	2	1	100%	Y	0.82	*	*			
		M69 Westbound	0	0	0	0%	Y	0.00	*	*			
		A5 Northbound	6	6	0	0%	Y	0.00	*	*			
		B4109 Rugby Road	2	2	0	0%	Y	0.00	*	*			
		M69 Eastbound	2	3	1	50%	Y	0.63	*	*			
		A5 Southbound	1	1	0	0%	Y	0.00	*	*			
	A5 Northbound	66	65	-1	-2%	Y	0.12	*	*				
	M69 Eastbound	B4109 Rugby Road	5	3	-2	-40%	Y	1.00	*	*			
		M69 Eastbound	286	288	2	1%	Y	0.06	*	*			
		A5 Southbound	34	34	0	0%	Y	0.00	*	*			
		B4109 Hinckley Road	1	1	0	0%	Y	0.00	*	*			
		B4109 Rugby Road	0	0	0	0%	Y	0.00	*	*			
		M69 Eastbound	32	31	-1	-3%	Y	0.18	*	*			
		A5 Southbound	91	90	-1	-1%	Y	0.11	*	*			
		B4109 Hinckley Road	2	2	0	0%	Y	0.00	*	*			
		M69 Westbound	30	30	0	0%	Y	0.00	*	*			
		A5 Southbound	2	2	0	0%	Y	0.00	*	*			
	AS/Wolvey Road	Wolvey Road Southbound	A5 Watling Street Eastbound	0	0	0	0%	N	0.00	*	*		
			Wolvey Road Southbound	1	0	-1	-100%	Y	1.41	*	*		
			Wolvey Road Southbound	0	0	0	0%	Y	0.00	*	*		
			A5 Watling Street Westbound	214	208	-6	-3%	Y	0.41	*	*		
			Wolvey Road Northbound	2	0	-2	-100%	Y	2.00	*	*		
		Wolvey Road Northbound	A5 Watling Street Westbound	0	0	0	0%	N	0.00	*	*		
			Wolvey Road Northbound	0	0	0	0%	N	0.00	*	*		
			A5 Watling Street Eastbound	0	0	0	0%	N	0.00	*	*		
			Wolvey Road Northbound	1	1	0	0%	Y	0.00	*	*		
A5 Watling Street Eastbound			150	151	1	1%	Y	0.08	*	*			
AS/Wolvey Road		A5 Watling Street Westbound	0	0	0	0%	Y	0.00	*	*			

APPENDIX 2: Journey Time Validation

		07:30 - 08:30										08:30 - 09:30										09:30 - 09:30											
Section	From	Description	To	Observed			Modelled			Validation				Modelled			Validation				Modelled			Validation									
				Dist.	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates		
1	Waiting Street (North of Wokey Rd)	-	Wokey Road	886m	67	59	87	104	1.1%	✓	✓	✓	✓	64	55	56	57	-0.1%	-3.3%	✓	✓	✓	✓	75	55	72	104	-4.1%	-5.3%	✓	✓	✓	✓
5	A5 Watling Rd SB - Jct Wokey	-	A5 Watling Rd SB - Jct MW RAB	394m	92	69	92	115	2.1%	✓	✓	✓	✓	72	45	47	50	-25.3%	-33%	✓	✓	✓	✓	62	45	70	115	-21.3%	-3.4%	✓	✓	✓	✓
15	RAB - Stoptime A5 Watling (N) to Rugby Rd	-		43m	7	4	5	5	-36%	✗	✓	✓	✓	5	4	4	4	-129%	-	✓	✓	✓	6	4	5	5	-233%	-	✓	✓	✓	✓	
16	RAB Circ Rugby Rd	-		71m	13	14	21	24	8.6%	✗	✓	✓	✓	9	8	9	11	0.1%	3%	✓	✓	✓	11	8	15	24	4.36%	-	✓	✓	✓	✓	
17	RAB Rugby Road Jct to M69 NB Onslp	-		15m	2	5	8	8	4.16%	✗	✓	✓	✓	2	4	4	4	2.14%	-	✓	✓	✓	2	4	5	8	3.145%	-	✓	✓	✓	✓	
18	RAB Circ M69 N On to M69 S Off	-		68m	10	14	15	16	8.45%	✗	✓	✓	✓	9	12	12	13	3.36%	-	✓	✓	✓	10	12	13	16	4.41%	-	✓	✓	✓	✓	
19	RAB M69 SB Off to Watling Rd SB	-		13m	2	2	2	2	0.6%	✗	✓	✓	✓	2	2	2	2	0.5%	-	✓	✓	✓	2	2	2	2	0.6%	-	✓	✓	✓	✓	
9	A5 Watling Rd SB MW RAB (S)	-	A5 Watling Rd SB (S)	743m	45	44	44	45	-2.3%	✗	✓	✓	✓	47	45	45	45	-2.4%	-	✓	✓	✓	46	44	45	45	-2.4%	-	✓	✓	✓	✓	
TOTAL	Waiting Street (North of Wokey Rd)	-	A5 Watling Rd SB (S)	2446m	259	200	273	316	14.6%	✗	✓	✓	✓	210	173	179	186	-31.1%	-1.51%	✓	✓	✓	234	172	226	317	-6.4%	-	✓	✓	✓	✓	

		07:30 - 08:30										08:30 - 09:30										16:30 - 18:30										
Section	From	Description	To	Observed			Modelled			Validation				Modelled			Validation				Modelled			Validation								
				Dist.	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	
10	A5 Watling Rd NB (S)	-	A5 Watling Rd NB MW RAB (S)	748m	102	85	90	92	-12.1%	✗	✓	✓	✓	78	86	91	96	13.16%	-	✓	✓	✓	90	85	90	96	0.0%	-	✓	✓	✓	✓
21	RAB Watling S to Hinckley Rd SB	-		27m	2	2	2	2	0.6%	✗	✓	✓	✓	2	2	2	2	0.1%	-	✓	✓	✓	2	2	2	2	0.2%	-	✓	✓	✓	✓
22	RAB Circ Hinckley	-		82m	6	7	8	8	2.25%	✗	✓	✓	✓	6	6	7	7	0.7%	-	✓	✓	✓	6	6	7	9	1.16%	-	✓	✓	✓	✓
23	RAB Hinckley Rd to M69 SB	-		16m	2	1	1	1	0.7%	✗	✓	✓	✓	1	1	1	1	0.13%	-	✓	✓	✓	1	1	1	1	0.2%	-	✓	✓	✓	✓
24	RAB Circ M69 S	-		104m	17	12	13	14	-21%	✗	✓	✓	✓	13	12	12	12	-1.7%	-	✓	✓	✓	15	12	13	14	-2.5%	-	✓	✓	✓	✓
25	RAB Circ M69 N On to M69 S Off	-		34m	4	3	3	3	-36%	✗	✓	✓	✓	5	3	3	3	-40%	-	✓	✓	✓	5	3	3	3	-38%	-	✓	✓	✓	✓
6	Rugby Road NB - Jct MW RAB	-	Rugby Road NB - Jct Sml RAB	382m	39	38	39	39	0.0%	✗	✓	✓	✓	39	37	39	41	-1.1%	-	✓	✓	✓	39	37	39	41	0.1%	-	✓	✓	✓	✓
2	Rugby Road NB - Jct Sml RAB	-	A5 Watling Rd NB - Field 2	904m	56	60	60	60	4.8%	✗	✓	✓	✓	56	59	59	59	3.6%	-	✓	✓	✓	56	59	59	60	4.7%	-	✓	✓	✓	✓
TOTAL	A5 Watling Rd NB (S)	-	A5 Watling Rd NB - Field 2	2299m	227	208	216	221	-12.1%	✗	✓	✓	✓	200	206	213	221	13.7%	-	✓	✓	✓	214	205	215	226	1.0%	-	✓	✓	✓	✓

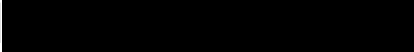
		07:30 - 08:30										08:30 - 09:30										16:30 - 18:30										
Section	From	Description	To	Observed			Modelled			Validation				Modelled			Validation				Modelled			Validation								
				Dist.	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	
31	M69 NB - Powerlines	-	M69 NB Offslp	687m	28	31	31	32	3.1%	✓	✓	✓	✓	29	31	31	31	3.9%	-	✓	✓	✓	29	31	31	32	3.1%	-	✓	✓	✓	✓
32	M69 NB Onslp	-	M69 NB Under	977m	27	31	31	31	4.1%	✓	✓	✓	✓	31	34	34	34	1.0%	-	✓	✓	✓	31	34	34	34	1.2%	-	✓	✓	✓	✓
33	M69 NB Onslp	-	M69 NB XXX	1724m	56	60	61	61	8.1%	✓	✓	✓	✓	55	60	60	60	9.5%	-	✓	✓	✓	55	60	60	61	9.9%	-	✓	✓	✓	✓
TOTAL	M69 NB - Powerlines	-	0 M69 NB XXX	3588m	115	125	126	127	11.0%	✓	✓	✓	✓	115	125	125	126	11.9%	-	✓	✓	✓	115	125	126	127	11.9%	-	✓	✓	✓	✓

		07:30 - 08:30										08:30 - 09:30										16:30 - 18:30										
Section	From	Description	To	Observed			Modelled			Validation				Modelled			Validation				Modelled			Validation								
				Dist.	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	
34	M69 SB XXX	-	M69 SB Onslp	1688m	56	67	67	67	9.1%	✓	✓	✓	✓	60	66	66	67	8.1%	-	✓	✓	✓	59	66	67	67	9.1%	-	✓	✓	✓	✓
35	M69 SB Onslp	-	M69 SB Onslp	871m	27	31	31	31	4.1%	✓	✓	✓	✓	28	30	31	31	1.0%	-	✓	✓	✓	27	30	31	31	1.2%	-	✓	✓	✓	✓
36	M69 SB Onslp	-	M69 SB Powerlines	857m	28	31	31	32	3.2%	✓	✓	✓	✓	28	31	31	31	3.9%	-	✓	✓	✓	29	31	31	32	3.1%	-	✓	✓	✓	✓
TOTAL	M69 SB XXX	-	0 M69 SB Powerlines	3588m	113	129	129	130	16.1%	✓	✓	✓	✓	116	127	128	129	12.1%	0.0%	✓	✓	✓	116	127	129	130	14.1%	0.0%	✓	✓	✓	✓

		07:30 - 08:30										08:30 - 09:30										16:30 - 18:30										
Section	From	Description	To	Observed			Modelled			Validation				Modelled			Validation				Modelled			Validation								
				Dist.	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	Avg.	Min.	Avg.	Max.	Actual %	% Diff.	Within 15%	Within 1 min.	Validates	
28	M69 SB Onslp	-	M69 SB Onslp	333m	50	49	52	53	2.4%	✓	✓	✓	✓	42	41	44	46	1.3%	-	✓	✓	✓	46	41	45	53	2.3%	-	✓	✓	✓	✓
19	RAB M69 SB Off to Watling Rd SB	-		13m	2	2	2	2	0.6%	✗	✓	✓	✓	2	2	2	2	0.5%	-	✓	✓	✓	2	2	2	2	0.6%	-	✓	✓	✓	✓
20	RAB CIRC Watling Road	-		103m	16	14	14	14	-2.1%	✗	✓	✓	✓	15	13	14	14	-2.1%	-	✓	✓	✓	15	13	14	14	-2.1%	-	✓	✓	✓	✓
21	RAB Watling S to Hinckley Rd SB	-		27m	2	2	2	2	0.6%	✗	✓	✓	✓	2	2	2	2	0.3%	-	✓	✓	✓	2	2	2	2	0.7%	-	✓	✓	✓	✓
22	RAB Circ Hinckley	-		82m	6	7	8	8	2.25%	✗	✓	✓	✓	6	6	7	7	0.7%	-	✓	✓	✓	6	6	7	9	1.16%	-	✓	✓	✓	✓
23	RAB Hinckley Rd to M69 SB	-		16m	2	1	1	1	0.7%	✗	✓	✓	✓	1	1	1	1	0.13%	-	✓	✓	✓	1	1	1	1	0.2%	-	✓	✓	✓	✓
24	RAB Circ M69 S	-		104m	17	12	13	14	-21%	✗	✓	✓	✓	13	12	12	12	-1.7%	-	✓	✓	✓	15	12	13	14	-2.5%	-	✓	✓	✓	✓
25	RAB M69 NB Off to Watling N	-		34m	4	3	3	3	-36%	✗	✓	✓	✓	5	3	3	3	-40%	-	✓	✓	✓	5	3	3	3	-38%	-	✓	✓	✓	✓
6	Rugby Road NB - Jct MW RAB	-	A5 Watling Rd NB - Jct Wokey	382m	39	38	39	39	0.0%	✗	✓	✓	✓	39	37	39	41	-1.1%	-	✓	✓	✓	39	37	39	41	0.1%	-	✓	✓	✓	✓
TOTAL	M69 SB Onslp	-	A5 Watling Rd NB - Jct Wokey	1095m	137	129	134	137	-1.3%	✗	✓ </																					



BETTER SOLUTIONS, INTELLIGENTLY ENGINEERED



Project Name	Hinckley National Rail Freight Interchange		
Document Number	HNRFI-BWB-GEN-XX-RP-TR-0005-M69 J2 Updated LMVR	BWB Ref	NTT2814
Author	DB	Status	S4
Checked	LB	Revision	P01
Approved	LB	Date	05/01/2021

1. INTRODUCTION

- 1.1 This Technical Note (TN) details microsimulation modelling undertaken by BWB to review and update an existing M69 J2 VISSIM model in support of the proposed National Rail Freight Interchange (NRFI) in Hinckley, Leicestershire.
- 1.2 The proposed development location is shown in **Figure 1**.

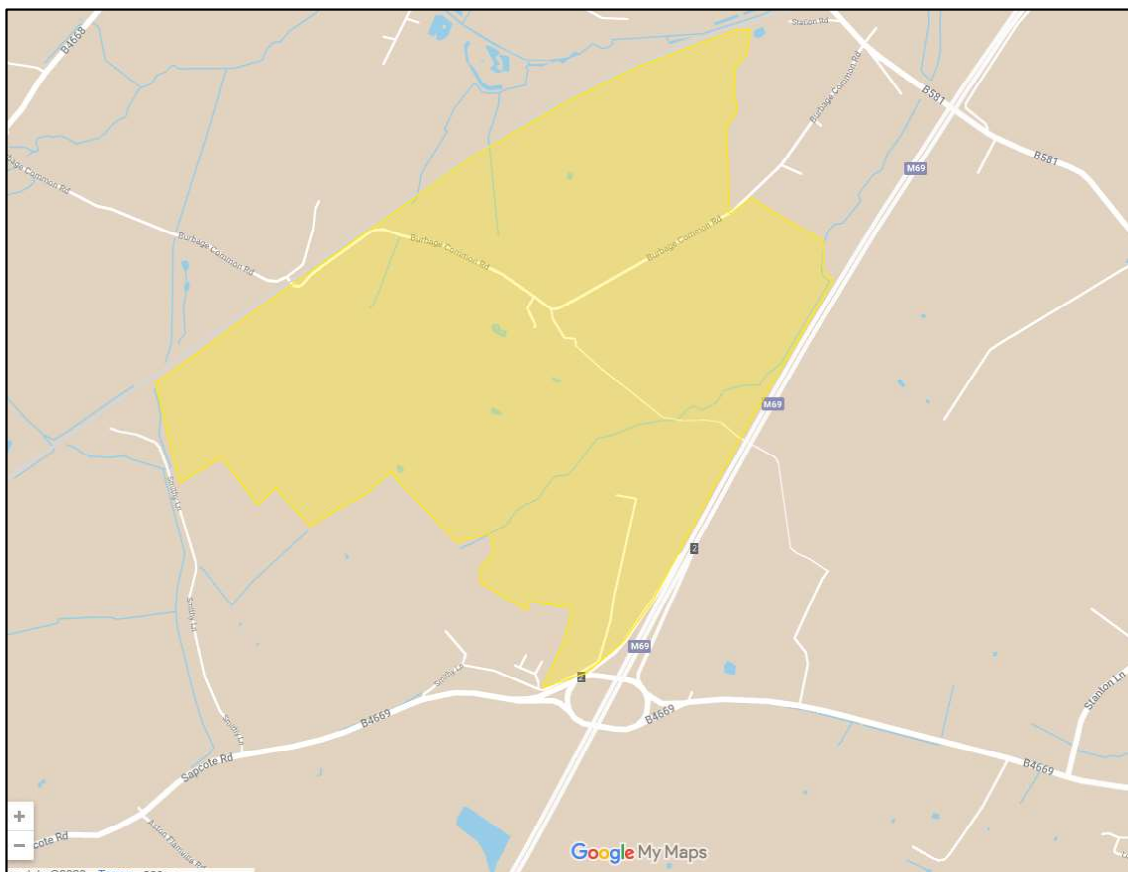


Figure 1 – Proposed NRFI Site Location

2. EXISTING M69 J2 VISSIM MODEL

- 2.1 BWB have received a VISSIM model of M69 J2 that has been approved for use by both Highways England (HE) and Leicestershire County Council (LCC). The extents of the model are shown in **Figure 2**.

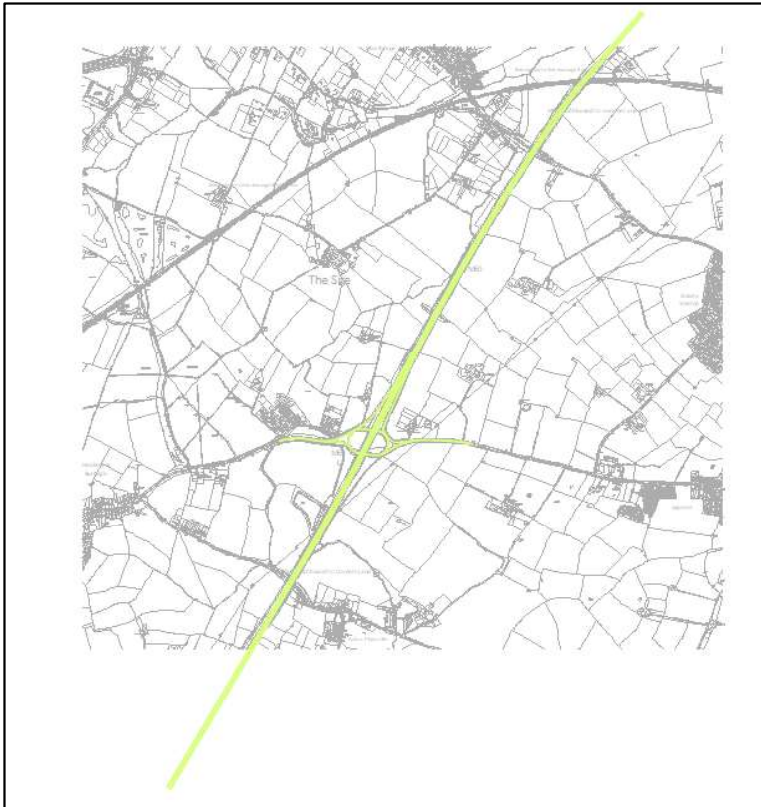


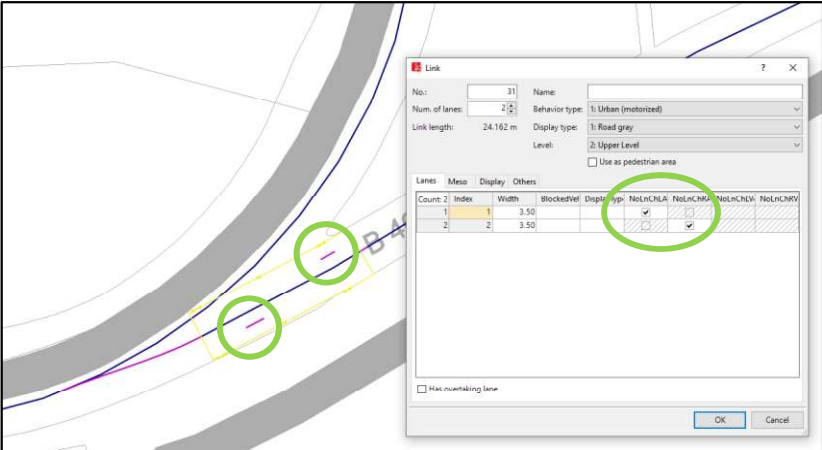

Figure 2 – M69 J2 VISSIM Model

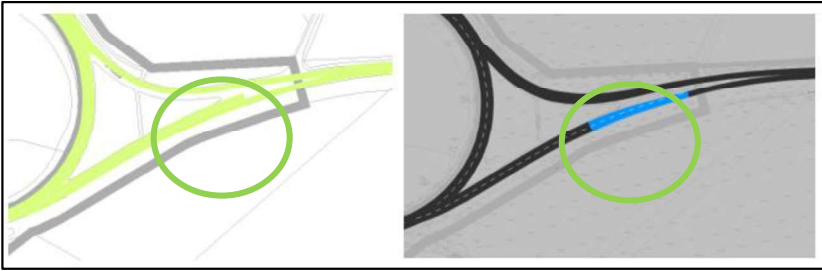

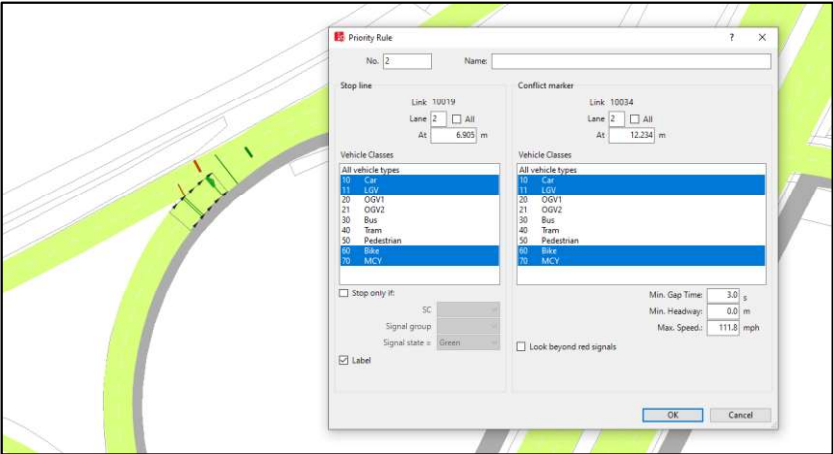
- 2.2 However, before this model has been adopted for use, a review of the model has been undertaken. This has been done to ensure that BWB are content with the model build and levels of calibration and validation prior to being used in support of the Hinckley NRFI.

Model Review

- 2.3 BWB's review of the M69 J2 model has identified a number of elements within the model which could be further improved upon. These would help both with the base model calibration and validation and the future year testing.
- 2.4 The elements for further improvement are detailed in **Table 1**.

Table 1 – Existing Model Elements to Improve

No.	Model Element	Comments
1	Links / Connectors	<p>Use of 'Right-Side Rule' on the circulatory links – these should be set to 'Urban (motorised)'</p> <p>Link/Connector set-up on the South-East and North-West sections where the approaches meet the circulatory could be improved by removing the mix of 'no lane change' on links and use of small connectors to control lane usage and on the North-West approach, ensuring the connectors match the lane markings. Examples of these elements are shown below.</p>  <p>Mix of 'No Lane Change' & Small Connectors for Lane Use</p>  <p>Small Connectors for Lane Use & Connector Set-Up on NW Approach</p> <p>The flares and merges in the model can be improved by the use of the 0.1m lane method. This provides a more natural progression of vehicles when either merging or diverging. An example of this change is shown below.</p>

		
2	Reduced Speed Areas	<p>There are a number of long Reduced Speed Areas on the circulatory, which have been used to control the speed of vehicles. RSAs should be limited to smaller sections and the use of Desired Speed Decision markers is considered a better form of vehicle speed over long sections.</p> 
3	Priority Rules	<p>The configuration of the priority rules are different to normal modelling practice, with the conflict markers being applied to specific vehicles, rather than 'All Vehicles'. An example for Light Vehicles on the B4669 West approach is shown below. To ensure a more simplified approach (and to avoid any potential issues in the future year modelling), the priority rules will be split into Lights and Heavies and the conflict markers will apply to 'All Vehicles'.</p> 

4	Journey Time Validation	Journey time validation has been based on a WiFi survey, undertaken in November 2017. However, within the Local Model Validation Report (LMVR), no details are provided on the sample size of these surveys. To reduce any risks in using this data, historic TomTom data has been obtained and a new journey time validation exercise will be undertaken to ensure the model validates against these times.
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3. UPDATED M69 J2 VISSIM MODEL

- 3.1 As a result of the model review undertaken, an updated version of the M69 J2 model has been developed that addresses the modelling elements noted.
- 3.2 These changes have also been made in line with a new VISSIM model being developed for M69 J1, as part of the same project. This has ensured consistency between the models in terms of the base data and parameters used.

Model Changes

- 3.3 A detailed breakdown of all of the changes made to the model is provided in the VISSIM Log in **Appendix A**. As a summary, the main changes in the model include:
 - The model has been updated to the latest stable version of VISSIM – VISSIM 2020 (SP09).
 - The PM period simulated has been amended to 1600-1900hrs, with 1630-1730hrs and 1730-1830hrs identified as the network-wide peak hours. As a result, updated Vehicle Inputs and Static Route flows have been calculated to suit the new simulation period.
 - A number of Base Data elements have been updated to match the M69 J1 model. These notably include 2D/3D models, Time/Desired Speed and 2D/3D Model Distributions, Driver Behaviour, Vehicle Types/Classes/Compositions and Link Behaviour Types.
 - The link and connector structure on the circulatory has been reviewed and updated based on OS and Topographical mapping. This has included a review of the B4669 West approach to better suit the lane markings and the inclusion of separate connectors on the M69 North and B4669 East approaches, to help with realistic lane use both on the approach and the circulatory. The behaviour has also been amended from 'Right-Side Rule' to 'Urban (motorised)' on the circulatory links as this is considered a more suitable behaviour.
 - An update of the Reduced Speed Areas (RSAs) has been undertaken. RSAs now only feature on the junction approaches and the B4669 East and West exit bends. The circulatory speed is now controlled by new Desired Speed Decision markers on the M69 and B4669 entries to the circulatory.
 - The Priority Rules (PRs) have been reconfigured to a more conventional set-up, with gap times for Light and Heavy Vehicles of 3.0s and 3.5s respectively.
 - During the calibration and validation process, it was necessary to amend the Desired Speed Distributions for the 60mph National Speed Limit (Single Carriageway) and the 70mph Motorway profiles. This was a result of the

distributions leading to journey times which were too slow on certain sections of the network. Therefore, a new set of Speed Distributions (No. 10000-10006 & 11000-11005) have been created and used, where the lower speed bands have been removed (see **Appendix B** for the changes). This approach is considered reasonable as the very low speeds are unrealistic for a vehicle travelling in free-flow conditions for the given speed limits.

Model Specification

3.4 The specification for the updated M69 J2 VISSIM model is as follows:

- VISSIM Version – 2020 (SP09)
- Model Base Year – 2017
- Model Time Periods:
 - Weekday AM – 07:00-07:30 (warm-up), 07:30-09:30 (peak period/s), 09:30-10:00 (cool-down)
 - Weekday PM – 16:00-16:30 (warm-up), 16:30-18:30 (peak period/s), 18:30-19:00 (cool-down)
- Vehicle Types Used:
 - Cars
 - LGV
 - OGV1 & OGV2
 - Buses & Coaches
 - Motorcycles

Model Calibration

3.5 Having made a number of changes to the model, a recalibration exercise was required to ensure that the model still performed as observed traffic conditions. For this model, traffic flows and maximum (average) queue lengths have been used as calibration measures.

Traffic Flows

3.6 Manual Classified Count (MCC) surveys were previously undertaken on 23rd November 2017 at M69 J2 and these have been used to inform the flows in the model.

3.7 Link counts from 23rd November 2017 have been collected from HE's Webtris website at the following site locations:

- M69 Mainline (M69/8147A) – Northbound
- M69 Mainline (M69/8147B) – Southbound
- M69 J2 On-Slip (M69/8147K) – Northbound
- M69 J2 Off-Slip (M69/8147L) - Southbound

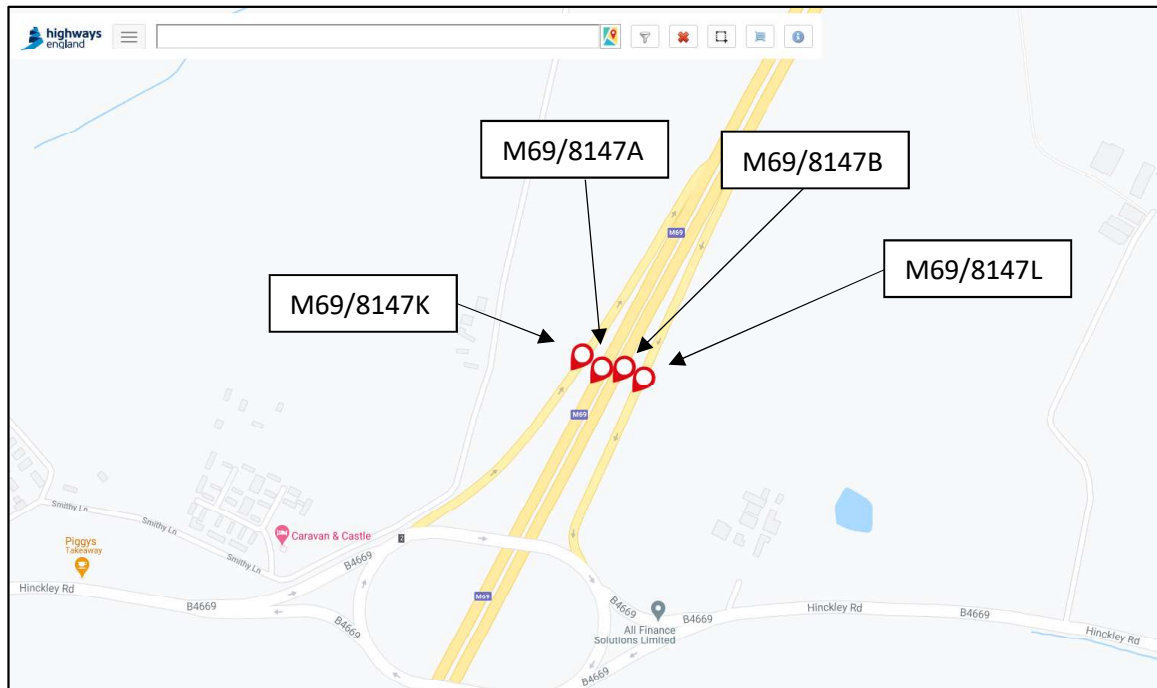


Figure 3 – Link Count Locations

- 3.8 For this model update, the M69 mainline flows have formed part of the flow calibration and the slip road flows were used a validation measure.
- 3.9 The process of flow calibration has involved iterations of minor adjustments to priority control and approach behaviour at key locations and on key routes. The calculated GEH statistic for the observed and modelled flows was considered for both the junction turning count and M69 mainline counts in accordance with the criteria stated in TAG Unit 3.1. To consider day to day variation in driver behaviour, the models were run, and results averaged over twenty random seeds. **Tables 2-7** summarise the flow calibration results for the AM and PM peak periods assessed.
- 3.10 For transparency, completeness and robustness, these results also include a comparison against the TfL criteria for key links, using a GEH value of 3 or under. The results show that all peak periods assessed achieves the ideal minimum 85% count, demonstrating that a strong flow calibration result has been achieved. A full breakdown of model calibration results can be found in **Appendix C**.

Table 2 – AM Flow Calibration – 0730-0830hrs

AM Peak (07:30-08:30) Summary - ALL	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 3 – AM Flow Calibration – 0830-0930hrs

AM Peak (08:30-09:30) Summary - ALL	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 4 – AM Flow Calibration – 0730-0930hrs

AM Peak (07:30-09:30) Summary - ALL	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 5 – PM Flow Calibration – 1630-1730hrs

PM Peak (16:30-17:30) Summary - ALL	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 6 – PM Flow Calibration – 1730-1830hrs

PM Peak (17:30-18:30) Summary - ALL	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 7 – PM Flow Calibration – 1630-1830hrs

PM Peak (17:30-18:30) Summary - ALL	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Queue Lengths

- 3.11 Queue length surveys were carried out at M69 J2 at the same time as the turning counts.
- 3.12 Queue comparisons are used as a calibration aid rather than validation criteria as a result of the subjective nature of human queue measurement within the survey data and the technical difference with how queue lengths are measured within VISSIM. This is particularly true when measuring queue lengths from a give-way line rather than from a signal stop line, as this will usually result in a harder to define rolling queue condition.
- 3.13 The observed queue lengths were recorded in vehicles for each approach; however, the survey data did not distinguish between light and heavy vehicles. Therefore, as the majority of the queuing vehicles were likely to be light vehicles, a factor of 6 has been applied to the survey data counts.
- 3.14 Queue lengths were output from the VISSIM model using the default queue criteria and average over 20 random seeds for both peak periods.
- 3.15 Overall, the results show that the modelled queues are broadly comparable with the observed data, although there are some variations. Some of this is down to inherent differences between the human onsite measures and software measures within the models, suggesting there is a subjective and difficult to define nature to queue length measurements.
- 3.16 Graphical comparisons of the maximum (average) queue lengths are provided in **Appendix D**.

Model Validation

- 3.17 As well as model calibration, a validation exercise has also been undertaken. This summarises the goodness of fit between modelled and observed outputs against independently collected data.
- 3.18 For this model, two independent datasets have been used:
 - M69 Slip Road Link Counts from HE's Webtris website
 - Journey Time data from TomTom

Link Validation

- 3.19 As detailed in Paragraph 3.7, slip road data from 23rd November 2017 has been collected and used as a flow validation measure. This has been subject to the same GEH statistic comparisons as detailed in TAG Unit 3.1, as well as a comparison against TfL's criteria for critical links. The results of the Slip Road Flow Validation are shown in **Tables 8-13**, with more detailed results provided in **Appendix E**.
- 3.20 The link validation shows that in both peak periods, the M69 off-slip flow falls outside of TfL's critical link GEH criteria of 3, but is still within 5 as required by TAG.

Table 8 – AM Link Validation – 0730-0830hrs

AM Peak (07:30-08:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	2
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 9 – AM Link Validation – 0830-0930hrs

AM Peak (08:30-09:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	2
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 10 – AM Link Validation – 0730-0930hrs

AM Peak (07:30-09:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	1
% of VISSIM counts with GEH <3	50.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 11 – PM Flow Validation – 1630-1730hrs

PM Peak (16:30-17:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	1
% of VISSIM counts with GEH <3	50.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 12 – PM Flow Validation – 1730-1830hrs

PM Peak (17:30-18:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	2
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 13 – PM Flow Validation – 1630-1830hrs

PM Peak (16:30-18:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	1
% of VISSIM counts with GEH <3	50.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Journey Time Validation

- 3.21 The journey time validation has been carried out using TomTom data collected for the network. This was chosen as it provides a high sample rate dataset which improves the overall robustness of the validation comparison.
- 3.22 The TomTom data is from April 2019, with 10th April 2019 the specific day-data. This was extracted for both M69 J1 and M69 J2. Whilst the data could have been extracted for November 2017 to tie in with the existing counts, the different dates allowed a more robust validation exercise to be undertaken.
- 3.23 The data is provided in small link sections, so for the purpose of providing journey time validation, multiple sections have been combined into longer journey routes, covering all major movements at key locations.

3.24 A total of 8 journey time routes have been prepared for the purpose of model validation. **Figure 4** shows the location of four, primary through routes. **Figure 5** shows the location of four secondary turning routes.

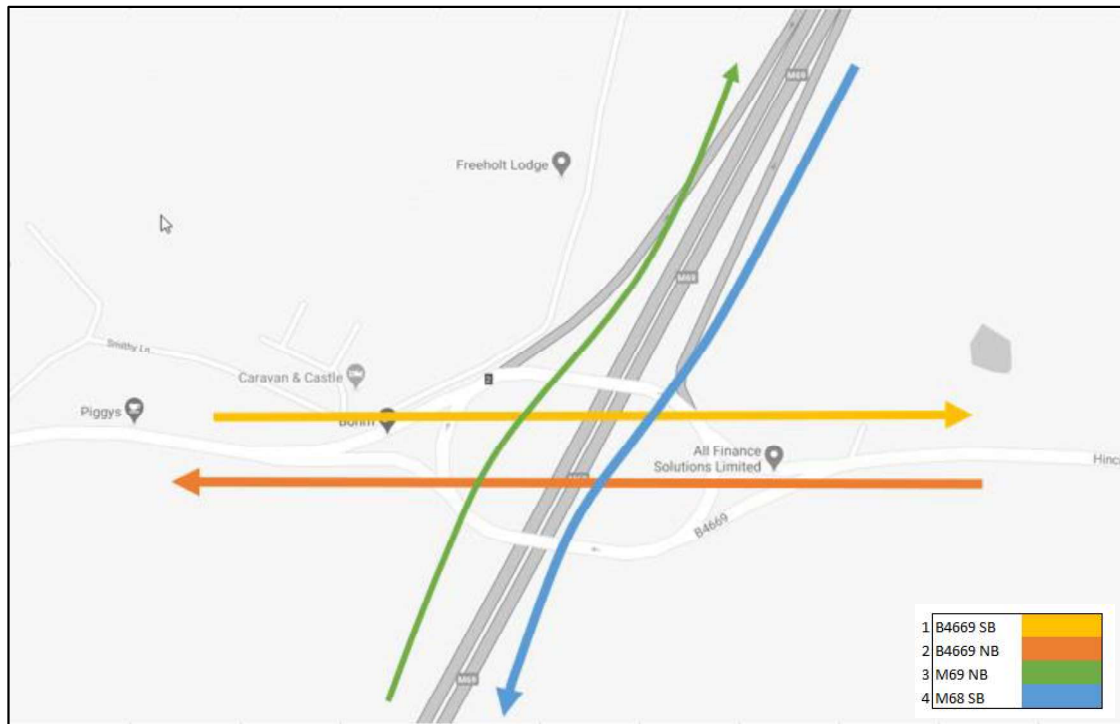


Figure 4 – TomTom Journey Time Routes – Primary Through Routes

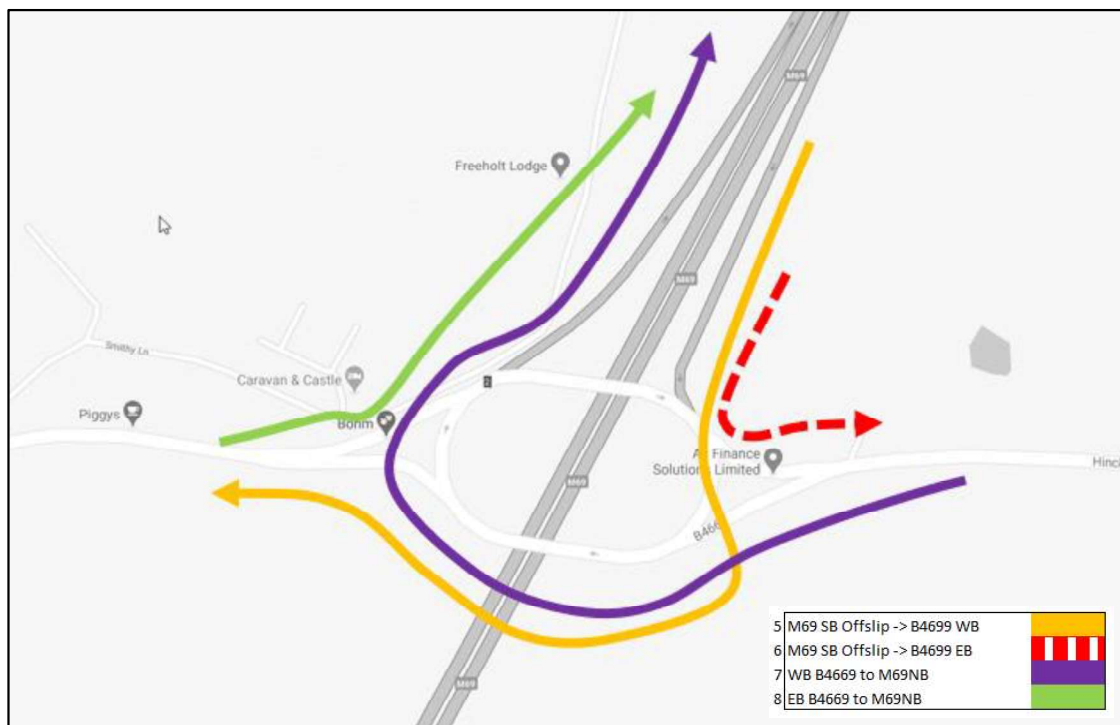


Figure 5 – TomTom Journey Time Routes – Secondary Turning Routes

- 3.25 **Tables 14 and 15** show the overall summary for all journey time routes and sections for the network. A more detailed breakdown of the journey time results can be found in **Appendix F**.

Table 14 – AM Journey Time Validation

Whole Routes	AM Peak		
	07:30-08:30	08:30-09:30	07:30-09:30
Criteria			
85% of measures within 15%	100%	100%	100%
85% of measures within 60 seconds	100%	100%	100%

Table 15 – PM Journey Time Validation

Whole Routes	PM Peak		
	16:30-17:30	17:30-18:30	16:30-18:30
Criteria			
85% of measures within 15%	88%	88%	88%
85% of measures within 60 seconds	100%	100%	100%

- 3.26 In accordance with TAG Unit 3.1 criteria, which recommends that the difference between observed and modelled journey times should be within 15% (or 1 minute if higher) for at least 85% of the routes evaluated (although that criteria is ideally designed for route sections over 3km and under 15km in length) it can be seen from **Tables 14 and 15** that both the AM and PM peak models meet the validation criteria.
- 3.27 In the AM peak, across the time periods 0730-0830hrs and 0830-0930hrs, 16/16 routes are within 15% and 60s. Across the full AM period of 0730-0930hrs, 8/8 routes are within 15% and 60s. In the PM peak, across the time periods 1630-1730hrs and 1730-1830hrs, 15/16 routes are within 15% and 16/16 routes are within 60s. The only route that is not within 15% is the M69 Northbound, where the model is slightly slower and has a 17% difference. Across the full PM period of 1630-1830hrs, 7/8 of the routes are within 15% and 8/8 routes are within 60s. The route that is again outside of the 15% difference is the M69 Northbound, where the difference is 17%. in total.
- 3.28 Overall, this is a very robust validation result, indicative of a good likeness between modelled performance and on-street conditions.

4. SUMMARY & CONCLUSIONS

Summary

- 4.1 This TN details the review of an existing M69 J2 VISSIM model and the subsequent model updates, recalibration and revalidation to demonstrate that the model is an accurate representation of the highway network within the study area and is fit for purpose for testing impacts associated with the proposed National Rail Freight Interchange (NRFI) in Hinckley, Leicestershire.

- 4.2 The updates to the model have been made to adopt more standard modelling practices, as well as to ensure the model is representative of on-site conditions and lane markings.
- 4.3 The recalibration and revalidation exercise has been undertaken to ensure that the changes made accurately reflects existing conditions with regards to:
- Traffic patterns;
 - Key junctions; and
 - Traffic volumes and routing.
- 4.4 In regard to the traffic turning and flow counts, the model exceeds the 85% criteria set by DMRB. Comparisons have also been made to queue lengths and these are broadly comparable with the observed data.
- 4.5 The model has also been validated to slip road flows and observed journey times within the extents of the network. Whilst the slip road flows fall outside of TfL's GEH criteria, they are within TAG's GEH criteria and are considered representative.
- 4.6 The journey time data has been provided as 15-minute intervals and an average one-hour journey time intervals have been used for each of the peak periods. The journey time comparisons show consistency between the modelled and observed journey time profiles across all of the journey time routes in the AM peak. In the PM peak, only one route out of the eight assessed falls outside of the 15% range. However, all of the routes are within 60s and as such, the journey times are considered representative.
- 4.7 Given that the traffic flows and journey times compare well with on-site conditions, it should be considered a successful calibration and validation exercise.

Conclusions

- 4.8 It is understood that as these conditions are met and are of a sufficient quality to represent real world conditions, the updated M69 Junction 2 VISSIM model is considered robust and acceptable for testing of the proposed development.

APPENDICES

APPENDIX 1: VISSIM Changes Log

Hinckley – M69 J2 – VISSIM MODEL UPDATE LOG

Base Network

2D/3D Models

- Updated to match M69 J1 model
 - o Added:
 - No. 16 – ‘LGV - Toyota Pickup’
 - No. 33 – ‘Bus - C2 Standard 2-doors left’
 - No. 64 – ‘Bike - Cycle Woman’
 - o Edited
 - No. 15 – ‘LGV – Ford Pickup’ – updated v3d reference
 - No. 17 – ‘Ford Van’ to ‘Chevy Van’ – updated v3d reference & axle position
 - No. 21 – ‘OGV1 - 2ax Rigid’ – updated v3d reference
 - No. 22 – ‘OGV1 - 3ax Rigid’ – updated v3d reference
 - No. 23 – ‘OGV2 - 3ax Articulated’ – updated v3d reference
 - No. 24 – ‘OGV2 - 4ax Articulated’ – updated v3d reference
 - No. 32 – ‘Bus – Coach’ – updated v3d reference

Distributions

- Time - Updated all distributions to match those in M69 J1 model
- Desired Speeds
 - o Updated 20mph, 30mph, 60mph [Single CWay] (except LGV), 70mph [MWay] – (except Bus) distributions to match those in M69 J1 model
 - o Added 50mph profiles – based on M69 J1 model
 - o Capping of 60mph [Single CWay] and 70mph [MWay] as part of Journey Time validation
- 2D/3D Model Distributions – Updated to match M69 J1 model
 - o Updated No. 11 - LGV
 - o Added No. 31 – Coach
 - o Updated No. 61 - Bike

Driving Behaviour

- Updated the following to match M69 J1 model:
 - o ‘Urban (motorised)’
 - Following, Lateral, Signal Control tabs
 - o ‘Right-side rule (motorized)’
 - Updated name to ‘Left-side rule (motorized)’, amended Lane Change, Signal Control tabs
 - o ‘Freeway (free lane selection)’
 - Lane Change, Signal Control
 - o ‘Footpath (no interaction)’
 - Lane Change, Signal Control
 - o ‘Cycle-Track (free overtaking)’
 - Lane Change, Signal Control
- Created the following behaviours, based on M69 J1 model:
 - o No. 6 – Urban (cyclists)
 - o No. 7 – Urban (Aggressive merge)
 - o No. 9 – Urban (aggressive merge)

Vehicle Types

- Updated Color Dist1, OccupDist and Capacity values to match M69 J1
- Added No. 310 – Coach

Vehicle Classes

- Created No. 22 HGV, No. 31 – Coach, No. 101 – Lights (Controls) and No. 102 Heavies (Controls)
- Updated VehTypes assignment to match M69 J1 model

Link Behaviour Types

- Added specific driver behaviour to Vehicle Classes for ‘Urban (motorized)’
- Renamed No. 2 – ‘M69 Left-side rule (motorized)’
- Added No. 6 – ‘Urban (merge/diverge)’

Display Types

- Updated No. 1 ‘Road (Urban) to match M69 J1 model
- Added No. 2 – ‘Road (Urban merge/diverge)’ and No. 3 – ‘Road (M69)’

Levels

- Added No. 3 – ‘Mapping’

Vehicle Compositions

- Added No. 22 – HGV, No. 31 – Coach, No. 61 – PCY

Background Images

- Removed current DWG and added:
 - o 07700-HYD-A-00-M2-D-0003 - OS.dwg
 - o 07700-HYD-A-00-M2-D-0006 - Topo.dwg

Links

- Updated M69 mainline and merge/diverge sections
 - o Better tie to mapping
 - o Updated to using 0.1m lane for merge/diverge sections
- Reconfigured approach and circulatory sections to match road markings
 - o Better tie to mapping
 - o Removal of small connectors and ‘No Lane Change’. Instead, split connectors on North and East approaches to control lane use
 - o Adjusted West approach to better match lane markings
- Link 10020 – amended from 20/150 to 5/200 (stop/lane change)
- Link 10018 – amended lane change from 100 to 200m
- Link 10052 – Lane change 200 to 120m
- Link 10018 – Lane change 200 to 120m
- Link 10071 – Lane change 200 to 90m
- Link 10076 – Lane change 200 to 90m
- Link 10068 – Lane change 200 to 90m
- Link 10073 – Lane change 200 to 90m

Desired Speed Decisions

- Updated all markers to include Coach
- Added new 40mph speed markers on circulatory entry and 60mph speed markers on East and West exits

Reduced Speed Areas

- Deleted all RSAs in the network
- Introduced new RSAs on approaches and on East and West exit bends

Priority Rules

- Deleted all PRs in the network
- Added new PRs, using 'Lights' and 'Heavies' as control – used default values of 3.0s and 3.5s

Vehicle Inputs

- Added names to inputs

Static Routes

- Added names to routes & repositioned points to start and end of network
- Added static routes for M69 N for MCY
- Updated static routes to account for 1600-1900hrs time period (with 1630-1830hrs assessment)
- Updated static routes to account for North and East approach connectors being split

Journey Time Markers

- Deleted current markers
- Added new markers to suit TomTom data locations (Sections 1-18)

Queue Markers

- Added new markers to North, East and West approaches at J2.

Data Collection Points

- Added new markers and measurements for TRADS data comparisons – M69 mainline and slip roads

Modification 1 – AM Peak

Vehicle Inputs

- Updated to match MG calcs

Static Routes

- Updated to match MG calcs
- Made adjustments to Route 16 for better validation of TRADS slip road flow

Modification 2 – PM Peak

Vehicle Inputs

- Updated to match MG calcs

Static Routes

- Updated to match MG calcs
- Made adjustments to Route 16 for better validation of TRADS slip road flow

APPENDIX 2: Desired Speed Distribution Changes

APPENDIX 3: Flow Calibration

AM Peak (07:30-08:30) Summary - ALL

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

TIMEINT
1800-5400
1800-5400
1800-5400
1800-5400
1800-5400
1800-5400
1800-5400
1800-5400

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2	M69 North	B4669 East	46	47	1	2%	Y	0.15	✓	✓			
		B4669 West	197	221	24	12%	Y	1.66	✓	✓			
	B4669 East	B4669 West	363	363	0	0%	Y	0.00	✓	✓			
		M69 North	226	224	-2	-1%	Y	0.13	✓	✓			
	B4669 West	M69 North	502	506	4	1%	Y	0.18	✓	✓			
		B4669 East	195	193	-2	-1%	Y	0.14	✓	✓			
M69	M69 Mainline	Northbound	2253	2259	6	0%	Y	0.13	✓	✓			
		Southbound	2847	2839	-8	0%	Y	0.15	✓	✓			

AM Peak (07:30-08:30) Summary - LIGHTS

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

TIMEINT
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Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2	M69 North	B4669 East	44	45	1	2%	Y	0.15	✓	✓			
		B4669 West	188	213	25	13%	Y	1.77	✓	✓			
	B4669 East	B4669 West	355	356	1	0%	Y	0.05	✓	✓			
		M69 North	216	213	-3	-1%	Y	0.20	✓	✓			
	B4669 West	M69 North	495	499	4	1%	Y	0.18	✓	✓			
		B4669 East	191	189	-2	-1%	Y	0.15	✓	✓			
M69	M69 Mainline	Northbound	2049	2054	5	0%	Y	0.11	✓	✓			
		Southbound	2599	2588	-11	0%	Y	0.22	✓	✓			

AM Peak (07:30-08:30) Summary - HEAVIES

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

TIMEINT
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Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2	M69 North	B4669 East	2	2	0	0%	Y	0.00	✓	✓			
		B4669 West	9	9	0	0%	Y	0.00	✓	✓			
	B4669 East	B4669 West	8	7	-1	-13%	Y	0.37	✓	✓			
		M69 North	10	11	1	10%	Y	0.31	✓	✓			
	B4669 West	M69 North	7	7	0	0%	Y	0.00	✓	✓			
		B4669 East	4	4	0	0%	Y	0.00	✓	✓			
M69	M69 Mainline	Northbound	204	205	1	0%	Y	0.07	✓	✓			
		Southbound	248	250	2	1%	Y	0.13	✓	✓			

AM Peak (08:30-09:30) Summary - ALL

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

TIMEINT
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Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2	M69 North	B4669 East	48	50	2	4%	N	0.29	✓	✓			
		B4669 West	182	198	16	9%	N	1.16	✓	✓			
	B4669 East	B4669 West	303	306	3	1%	N	0.17	✓	✓			
		M69 North	114	114	0	0%	N	0.00	✓	✓			
	B4669 West	M69 North	292	293	1	0%	N	0.06	✓	✓			
		B4669 East	173	175	2	1%	N	0.15	✓	✓			
M69	M69 Mainline	Northbound	1994	1993	-1	0%	N	0.02	✓	✓			
		Southbound	2302	2299	-3	0%	N	0.06	✓	✓			

AM Peak (08:30-09:30) Summary - LIGHTS

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

TIMEINT
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Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2	M69 North	B4669 East	38	39	1	3%	N	0.16	✓	✓			
		B4669 West	173	190	17	10%	N	1.26	✓	✓			
	B4669 East	B4669 West	291	294	3	1%	N	0.18	✓	✓			
		M69 North	105	106	1	1%	N	0.10	✓	✓			
	B4669 West	M69 North	287	288	1	0%	N	0.06	✓	✓			
		B4669 East	171	173	2	1%	N	0.15	✓	✓			
M69	M69 Mainline	Northbound	1727	1728	1	0%	N	0.02	✓	✓			
		Southbound	2044	2042	-2	0%	N	0.04	✓	✓			

AM Peak (08:30-09:30) Summary - HEAVIES

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

TIMEINT
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Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2	M69 North	B4669 East	10	11	1	10%	N	0.31	✓	✓			
		B4669 West	9	8	-1	-11%	N	0.34	✓	✓			
	B4669 East	B4669 West	12	12	0	0%	N	0.00	✓	✓			
		M69 North	9	9	0	0%	N	0.00	✓	✓			
	B4669 West	M69 North	5	5	0	0%	N	0.00	✓	✓			
		B4669 East	2	2	0	0%	N	0.00	✓	✓			
M69	M69 Mainline	Northbound	267	266	-1	0%	N	0.06	✓	✓			
		Southbound	258	257	-1	0%	N	0.06	✓	✓			

AM Peak (07:30-09:30) Summary - ALL

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2	M69 North	B4669 East	94	97	3	3%	N	0.31	✓	✓			
		B4669 West	379	419	40	11%	N	2.00	✓	✓			
	B4669 East	B4669 West	666	669	3	0%	N	0.12	✓	✓			
		M69 North	340	338	-2	-1%	N	0.11	✓	✓			
	B4669 West	M69 North	794	799	5	1%	N	0.18	✓	✓			
		B4669 East	368	368	0	0%	N	0.00	✓	✓			
M69	M69 Mainline	Northbound	4247	4252	5	0%	N	0.08	✓	✓			
		Southbound	5149	5138	-11	0%	N	0.15	✓	✓			

AM Peak (07:30-09:30) Summary - LIGHTS

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2	M69 North	B4669 East	82	84	2	2%	N	0.22	✓	✓			
		B4669 West	361	403	42	12%	N	2.15	✓	✓			
	B4669 East	B4669 West	646	650	4	1%	N	0.16	✓	✓			
		M69 North	321	319	-2	-1%	N	0.11	✓	✓			
	B4669 West	M69 North	782	787	5	1%	N	0.18	✓	✓			
		B4669 East	362	362	0	0%	N	0.00	✓	✓			
M69	M69 Mainline	Northbound	3776	3782	6	0%	N	0.10	✓	✓			
		Southbound	4643	4630	-13	0%	N	0.19	✓	✓			

AM Peak (07:30-09:30) Summary - HEAVIES

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2	M69 North	B4669 East	12	13	1	8%	N	0.28	✓	✓			
		B4669 West	18	17	-1	-6%	N	0.24	✓	✓			
	B4669 East	B4669 West	20	19	-1	-5%	N	0.23	✓	✓			
		M69 North	19	20	1	5%	N	0.23	✓	✓			
	B4669 West	M69 North	12	12	0	0%	N	0.00	✓	✓			
		B4669 East	6	6	0	0%	N	0.00	✓	✓			
M69	M69 Mainline	Northbound	471	471	0	0%	N	0.00	✓	✓			
		Southbound	506	507	1	0%	N	0.04	✓	✓			

PM Peak (16:30-17:30) Summary - ALL

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3,1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3,1 flow criteria	100.00%

TIMEINT
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Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 - 2700	>2700
M69 J2	M69 North	B4669 East	165	177	12	7%	Y	0.92	✓	✓			
		B4669 West	316	351	35	11%	Y	1.92	✓	✓			
	B4669 East	B4669 West	228	231	3	1%	Y	0.20	✓	✓			
		M69 North	22	22	0	0%	Y	0.00	✓	✓			
	B4669 West	M69 North	157	158	1	1%	Y	0.08	✓	✓			
		B4669 East	372	373	1	0%	Y	0.05	✓	✓			
M69	M69 Mainline	Northbound	2304	2313	9	0%	Y	0.19	✓	✓			
		Southbound	2197	2147	-50	-2%	Y	1.07	✓	✓			

PM Peak (16:30-17:30) Summary - LIGHTS

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3,1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3,1 flow criteria	100.00%

TIMEINT
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Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 - 2700	>2700
M69 J2	M69 North	B4669 East	158	169	11	7%	Y	0.86	✓	✓			
		B4669 West	293	327	34	12%	Y	1.93	✓	✓			
	B4669 East	B4669 West	227	230	3	1%	Y	0.20	✓	✓			
		M69 North	21	21	0	0%	Y	0.00	✓	✓			
	B4669 West	M69 North	157	158	1	1%	Y	0.08	✓	✓			
		B4669 East	369	370	1	0%	Y	0.05	✓	✓			
M69	M69 Mainline	Northbound	2154	2161	7	0%	Y	0.15	✓	✓			
		Southbound	2027	1977	-50	-2%	Y	1.12	✓	✓			

PM Peak (16:30-17:30) Summary - HEAVIES

Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3,1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3,1 flow criteria	100.00%

TIMEINT
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Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 - 2700	>2700
M69 J2	M69 North	B4669 East	7	9	2	29%	Y	0.71	✓	✓			
		B4669 West	23	24	1	4%	Y	0.21	✓	✓			
	B4669 East	B4669 West	1	1	0	0%	Y	0.00	✓	✓			
		M69 North	1	1	0	0%	Y	0.00	✓	✓			
	B4669 West	M69 North	0	0	0	0%	Y	0.00	✓	✓			
		B4669 East	3	3	0	0%	Y	0.00	✓	✓			
M69	M69 Mainline	Northbound	150	152	2	1%	Y	0.16	✓	✓			
		Southbound	170	170	0	0%	Y	0.00	✓	✓			

PM Peak (17:30-18:30) Summary - ALL	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

TIMEINT	Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
		Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
5400-9000	M69 J2	M69 North	B4669 East	181	188	7	4%	N	0.52	✓	✓			
5400-9000			B4669 West	340	355	15	4%	N	0.80	✓	✓			
5400-9000		B4669 East	B4669 West	179	180	1	1%	N	0.07	✓	✓			
5400-9000			M69 North	22	21	-1	-5%	N	0.22	✓	✓			
5400-9000		B4669 West	M69 North	142	147	5	4%	N	0.42	✓	✓			
5400-9000			B4669 East	308	306	-2	-1%	N	0.11	✓	✓			
5400-9000	M69	M69 Mainline	Northbound	2164	2168	4	0%	N	0.09	✓	✓			
5400-9000			Southbound	2240	2231	-9	0%	N	0.19	✓	✓			

PM Peak (17:30-18:30) Summary - LIGHTS	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

TIMEINT	Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
		Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
5400-9000	M69 J2	M69 North	B4669 East	175	182	7	4%	N	0.52	✓	✓			
5400-9000			B4669 West	315	330	15	5%	N	0.84	✓	✓			
5400-9000		B4669 East	B4669 West	177	178	1	1%	N	0.08	✓	✓			
5400-9000			M69 North	22	21	-1	-5%	N	0.22	✓	✓			
5400-9000		B4669 West	M69 North	140	145	5	4%	N	0.42	✓	✓			
5400-9000			B4669 East	308	306	-2	-1%	N	0.11	✓	✓			
5400-9000	M69	M69 Mainline	Northbound	2026	2031	5	0%	N	0.11	✓	✓			
5400-9000			Southbound	2045	2035	-10	0%	N	0.22	✓	✓			

PM Peak (17:30-18:30) Summary - HEAVIES	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

TIMEINT	Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
		Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
5400-9000	M69 J2	M69 North	B4669 East	6	6	0	0%	Y	0.00	✓	✓			
5400-9000			B4669 West	25	25	0	0%	Y	0.00	✓	✓			
5400-9000		B4669 East	B4669 West	2	2	0	0%	Y	0.00	✓	✓			
5400-9000			M69 North	0	0	0	-	Y	0.00	✓	✓			
5400-9000		B4669 West	M69 North	2	2	0	0%	Y	0.00	✓	✓			
5400-9000			B4669 East	0	0	0	-	Y	0.00	✓	✓			
5400-9000	M69	M69 Mainline	Northbound	138	137	-1	-1%	Y	0.09	✓	✓			
5400-9000			Southbound	195	196	1	1%	Y	0.07	✓	✓			

PM Peak (17:30-18:30) Summary - ALL	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3,1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3,1 flow criteria	100.00%

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 - 2700	>2700
M69 J2	M69 North	B4669 East	346	365	19	5%	N	1.01	✓	✓			
		B4669 West	656	706	50	8%	N	1.92	✓	✓			
	B4669 East	B4669 West	407	411	4	1%	N	0.20	✓	✓			
		M69 North	44	43	-1	-2%	N	0.15	✓	✓			
	B4669 West	M69 North	299	305	6	2%	N	0.35	✓	✓			
		B4669 East	680	679	-1	0%	N	0.04	✓	✓			
M69	M69 Mainline	Northbound	4468	4481	13	0%	N	0.19	✓	✓			
		Southbound	4437	4378	-59	-1%	N	0.89	✓	✓			

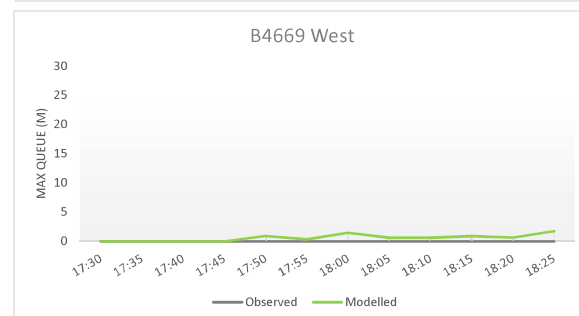
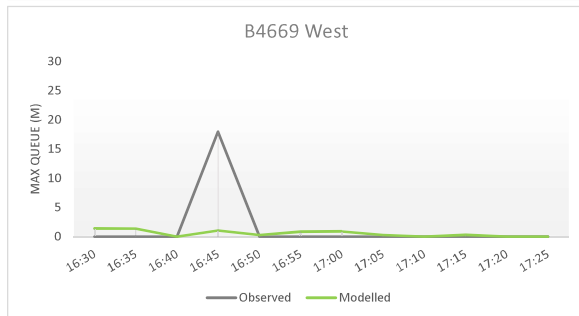
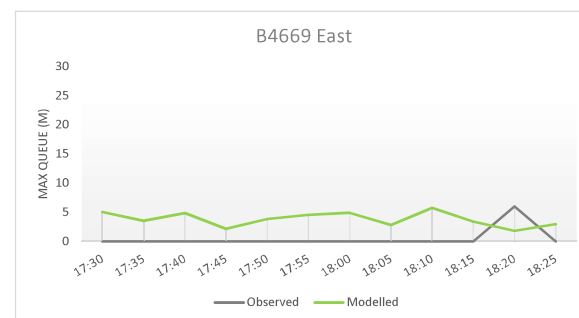
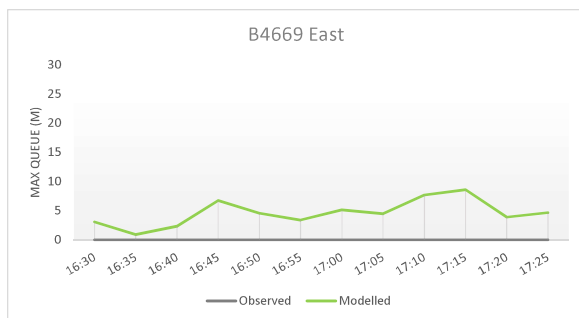
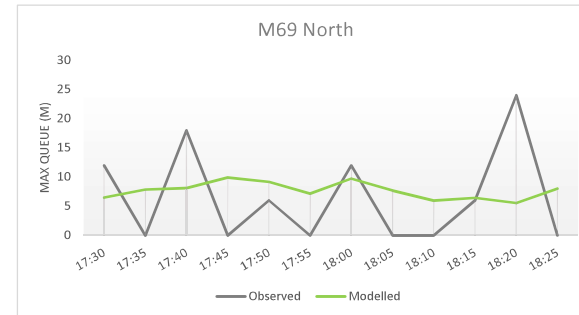
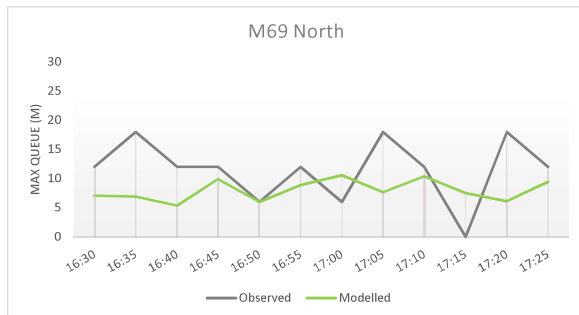
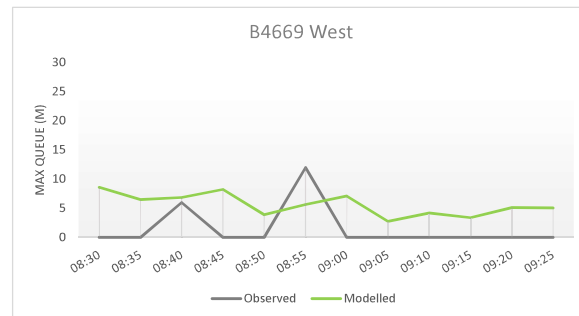
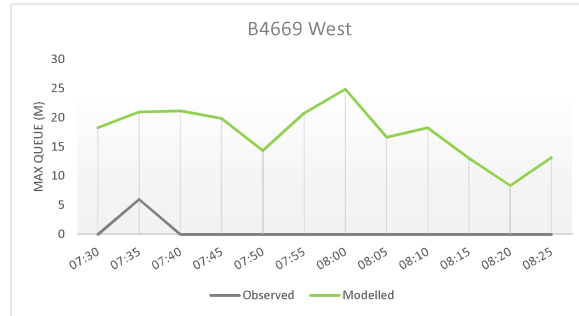
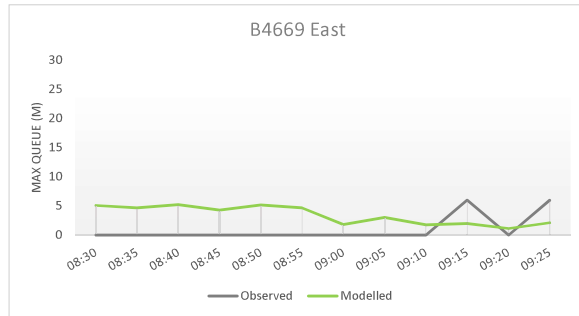
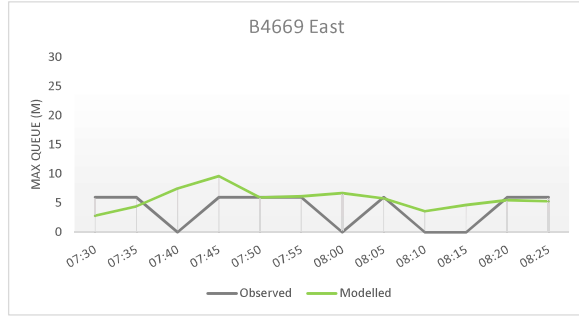
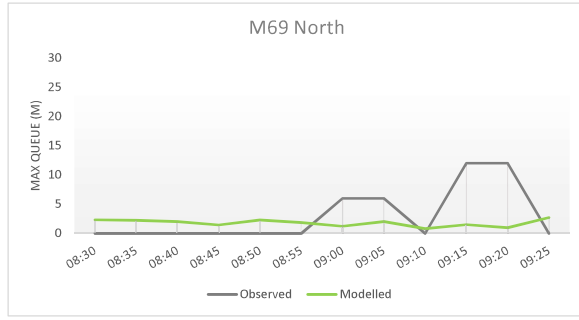
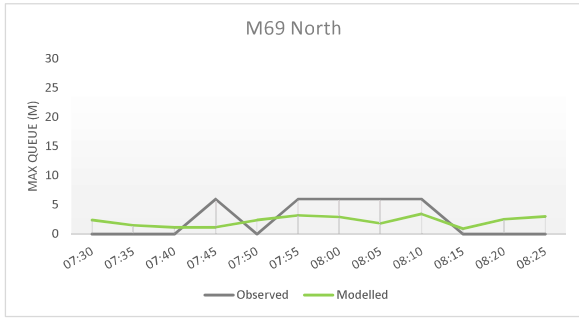
PM Peak (17:30-18:30) Summary - LIGHTS	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3,1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3,1 flow criteria	100.00%

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 - 2700	>2700
M69 J2	M69 North	B4669 East	333	351	18	5%	N	0.97	✓	✓			
		B4669 West	608	657	49	8%	N	1.95	✓	✓			
	B4669 East	B4669 West	404	408	4	1%	N	0.20	✓	✓			
		M69 North	43	42	-1	-2%	N	0.15	✓	✓			
	B4669 West	M69 North	297	303	6	2%	N	0.35	✓	✓			
		B4669 East	677	676	-1	0%	N	0.04	✓	✓			
M69	M69 Mainline	Northbound	4180	4192	12	0%	N	0.19	✓	✓			
		Southbound	4072	4012	-60	-1%	N	0.94	✓	✓			

PM Peak (17:30-18:30) Summary - HEAVIES	
Total number of counts considered	8
VISSIM model counts with GEH <3	8
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	8
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	8
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3,1 criteria	8
% of VISSIM counts meeting WebTAG Unit 3,1 flow criteria	100.00%

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 - 2700	>2700
M69 J2	M69 North	B4669 East	13	15	2	15%	Y	0.53	✓	✓			
		B4669 West	48	49	1	2%	Y	0.14	✓	✓			
	B4669 East	B4669 West	3	3	0	0%	Y	0.00	✓	✓			
		M69 North	1	1	0	0%	Y	0.00	✓	✓			
	B4669 West	M69 North	2	2	0	0%	Y	0.00	✓	✓			
		B4669 East	3	3	0	0%	Y	0.00	✓	✓			
M69	M69 Mainline	Northbound	288	289	1	0%	Y	0.06	✓	✓			
		Southbound	365	366	1	0%	Y	0.05	✓	✓			

APPENDIX 4: Queue Comparisons



PM Peak (16:30-17:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	1
% of VISSIM counts with GEH <3	50.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Link	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
	M69 J2 Off-Slip	Southbound	601	529	-72	-12%	Y	3.03	*	✓			
	M69 J2 On-Slip	Northbound	182	180	-2	-1%	Y	0.15	✓	✓			

TIMEINT
1800-5400
1800-5400

PM Peak (17:30-18:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	2
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Link	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2 Off-Slip	Southbound		610	542	-68	-11%	Y	2.83	✓	✓			
M69 J2 On-Slip	Northbound		175	167	-8	-5%	Y	0.61	✓	✓			

TIMEINT
5400-9000
5400-9000

PM Peak (16:30-18:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	1
% of VISSIM counts with GEH <3	50.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Junction/ Movement	Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met					
	Link	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69 J2 Off-Slip	Southbound		1211	1071	-140	-12%	Y	4.14	*	✓			
M69 J2 On-Slip	Northbound		357	347	-10	-3%	Y	0.53	✓	✓			

APPENDIX 5: Flow Validation

AM Peak (07:30-08:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	2
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69	M69 J2 Off-Slip	Southbound	316	271	-45	-14%	Y	2.63	✓	✓			
	M69 J2 On-Slip	Northbound	732	730	-2	0%	Y	0.07	✓	✓			

TIMEINT
1800-5400
1800-5400

AM Peak (08:30-09:30) Summary - ALL	
Total number of counts considered	2
VISSIM model counts with GEH <3	2
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69	M69 J2 Off-Slip	Southbound	281	247	-34	-12%	Y	2.09	✓	✓			
	M69 J2 On-Slip	Northbound	412	408	-4	-1%	Y	0.20	✓	✓			

TIMEINT
5400-9000
5400-9000

AM Peak (07:30-09:30) Summary - ALL

Total number of counts considered	2
VISSIM model counts with GEH <3	1
% of VISSIM counts with GEH <3	50.00%
VISSIM model counts with GEH <5	2
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	2
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	2
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 – 2700	>2700
M69	M69 J2 Off-Slip	Southbound	597	518	-79	-13%	Y	3.35	X	✓			
	M69 J2 On-Slip	Northbound	1144	1138	-6	-1%	Y	0.18	✓	✓			

APPENDIX 6: Journey Time Validation

Section	Description	Observed			Modelled			Validation			Modelled			Validation								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	Avg.	Min.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	
1	B4699 WB Sapote Rd - Woodside Garage	530m	27	29	29	29	29	29	29	29	0	0%	✓	✓	27	29	29	0	0%	✓	✓	✓
2	B4699 WB Sapote Rd - Smyth Ln	416m	26	25	26	26	26	26	26	26	0	0%	✓	✓	26	25	26	0	0%	✓	✓	✓
5	RAB B4699 EB	1289m	73	63	63	63	63	63	63	63	0	-13%	✓	✓	67	62	63	0	-5%	✓	✓	✓
TOTAL	B4699 WB Sapote Rd - Woodside Garage	2274m	127	118	120	121	121	117	117	118	0	0%	✓	✓	120	117	119	0	-2%	✓	✓	✓

Section	Description	Observed			Modelled			Validation			Modelled			Validation								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	Avg.	Min.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	
3	B4699 WB Sapote Rd - Woodside Garage	129m	71	67	67	68	68	66	66	67	0	0%	✓	✓	69	66	67	0	-3%	✓	✓	✓
6	B4699 WB Sapote Rd - Smyth Ln	234m	27	25	25	25	25	25	25	25	-2	-8%	✓	✓	30	25	25	-5	-17%	✓	✓	✓
4	B4699 WB Sapote Rd - Smyth Ln	487m	28	28	28	28	28	28	28	28	-2	-4%	✓	✓	29	27	28	-2	-8%	✓	✓	✓
TOTAL	B4699 WB Sapote Rd - Woodside Garage	2077m	127	120	120	121	121	118	118	119	-9	-7%	✓	✓	128	118	119	-10	-8%	✓	✓	✓

Section	Description	Observed			Modelled			Validation			Modelled			Validation								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	Avg.	Min.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	
15	M69 NB Lychgate Ln	2160m	69	80	80	80	80	80	80	80	11	16%	✓	✓	69	80	80	11	16%	✓	✓	✓
16	M69 NB End	633m	20	22	22	22	22	22	22	22	3	13%	✓	✓	20	22	22	3	13%	✓	✓	✓
TOTAL	M69 NB Lychgate Ln	2813m	89	103	103	103	103	103	103	103	14	15%	✓	✓	89	102	103	14	15%	✓	✓	✓

Section	Description	Observed			Modelled			Validation			Modelled			Validation								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	Avg.	Min.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	
17	M69 SB Start	603m	20	22	22	22	22	22	22	22	2	8%	✓	✓	20	22	22	2	8%	✓	✓	✓
18	M69 SB Offlap	2197m	69	76	77	77	77	76	76	76	6	7%	✓	✓	70	76	76	6	9%	✓	✓	✓
TOTAL	M69 SB Lychgate Ln	2801m	89	98	99	99	98	97	97	98	8	7%	✓	✓	90	97	98	8	9%	✓	✓	✓

Section	Description	Observed			Modelled			Validation			Modelled			Validation								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	Avg.	Min.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	
13	NBOnslp	472m	21	21	21	21	21	21	21	21	0	0%	✓	✓	20	21	21	0	0%	✓	✓	✓
9	RAB Crc SB M69 Offlap to B4699	16m	1	1	1	1	1	1	1	1	0	-24%	✓	✓	1	1	1	0	-24%	✓	✓	✓
10	RAB Crc SB	111m	8	7	7	7	7	7	7	7	-8	-8%	✓	✓	8	7	7	-8	-8%	✓	✓	✓
11	RAB Crc WB	218m	14	12	12	12	12	12	12	12	-2	-15%	✓	✓	14	12	12	-2	-15%	✓	✓	✓
3	RAB 4699 EB Sapote Rd	234m	27	25	25	25	25	25	25	25	-8	-24%	✓	✓	33	25	25	-8	-24%	✓	✓	✓
TOTAL	NBOnslp	640m	71	65	66	66	66	65	65	66	-10	-16%	✓	✓	76	65	66	-10	-16%	✓	✓	✓

Section	Description	Observed			Modelled			Validation			Modelled			Validation								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	Avg.	Min.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	
13	NBOnslp	472m	21	21	21	21	21	21	21	21	0	0%	✓	✓	20	21	21	0	0%	✓	✓	✓
9	RAB Crc SB M69 Offlap to B4699	16m	1	1	1	1	1	1	1	1	0	-24%	✓	✓	1	1	1	0	-24%	✓	✓	✓
10	RAB Crc SB	111m	8	7	7	7	7	7	7	7	-8	-8%	✓	✓	8	7	7	-8	-8%	✓	✓	✓
11	RAB Crc WB	218m	14	12	12	12	12	12	12	12	-2	-15%	✓	✓	14	12	12	-2	-15%	✓	✓	✓
3	RAB 4699 EB Sapote Rd	234m	27	25	25	25	25	25	25	25	-8	-24%	✓	✓	33	25	25	-8	-24%	✓	✓	✓
TOTAL	NBOnslp	1288m	94	84	85	85	85	84	84	85	-1	-1%	✓	✓	84	85	85	-1	-1%	✓	✓	✓

Section	Description	Observed			Modelled			Validation			Modelled			Validation								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	Avg.	Min.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	
2	B4699 WB Sapote Rd - Smyth Ln	476m	26	26	26	26	26	26	26	26	0	0%	✓	✓	27	26	27	0	0%	✓	✓	✓
7	RAB Crc B4699 WB	15m	1	1	1	1	1	1	1	1	0	0%	✓	✓	1	1	1	0	0%	✓	✓	✓
14	SB Offlap	430m	24	23	23	23	23	23	23	23	0	-1%	✓	✓	23	23	23	0	-1%	✓	✓	✓
TOTAL	B4699 WB Sapote Rd - Smyth Ln - SB Offlap	446m	52	50	50	50	50	49	49	50	0	-1%	✓	✓	51	49	51	0	0%	✓	✓	✓

Section	Description	Observed			Modelled			Validation			Modelled			Validation								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	Avg.	Min.	Max.	Actual Diff. %	Diff.	Within 1 min.	Validates	
6	B4699 WB Sapote Rd - Woodside Garage	129m	71	67	67	68	68	66	66	67	0	0%	✓	✓	69	66	67	0	-3%	✓	✓	✓
11	RAB Crc WB	218m	14	12	12	12	12	12	12	12	-2	-15%	✓	✓	14	12	12	-2	-15%	✓	✓	✓
12	RAB Crc WB B4699 to EB	105m	7	9	9	9	9	9	9	9	3	37%	✓	✓	6	9	9	3	37%	✓	✓	✓
7	RAB Crc B4699 WB	15m	1	1	1	1	1	1	1	1	0	0%	✓	✓	1	1	1	0	0%	✓	✓	✓
14	SB Offlap	430m	24	23	23	23	23	23	23	23	0	-1%	✓	✓	23	23	23	0	-1%	✓	✓	✓
TOTAL	B4699 WB Sapote Rd - Woodside Garage	768m	116	111	112	113	110	110	111	111	0	0%	✓	✓	113	110	111	0	0%	✓	✓	✓

Section	Description	Observed			Modelled			Validation			16:30 - 17:30			17:30 - 18:30			18:30 - 19:30								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates
1	B4699 WB Sapote Rd - Woodside C	530m	29	29	29	29	29	29	29	29	0	0%	✓	28	29	29	0	0%	✓	28	29	29	0	0%	✓
2	B4699 WB Sapote Rd - Smyth Ln	476m	24	24	24	24	24	24	24	24	0	0%	✓	24	24	24	0	0%	✓	24	24	24	0	0%	✓
5	RAB B4699 EB	1289m	63	64	65	65	65	64	65	65	4	7%	✓	62	64	65	3	5%	✓	62	64	65	3	5%	✓
TOTAL	B4699 WB Sapote Rd - Woodside Garage	2274m	117	118	118	118	118	110	117	118	0	0%	✓	114	117	118	4	4%	✓	114	117	118	4	4%	✓

Section	Description	Observed			Modelled			Validation			16:30 - 17:30			17:30 - 18:30			18:30 - 19:30								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates
6	B4699 WB Sapote Rd - Woodside C	530m	29	29	29	29	29	29	29	29	0	0%	✓	28	29	29	0	0%	✓	28	29	29	0	0%	✓
3	RAB 4699 EB Sapote Rd	234m	24	25	25	25	25	24	25	25	1	5%	✓	24	25	25	1	5%	✓	24	25	25	1	5%	✓
4	B4699 WB Sapote Rd - Smyth Ln	487m	28	28	28	28	28	27	27	28	0	0%	✓	27	27	28	0	0%	✓	27	27	28	0	0%	✓
TOTAL	B4699 WB Sapote Rd - Woodside Garage	2072m	118	117	118	118	118	115	117	118	2	2%	✓	116	117	118	1	1%	✓	116	117	118	1	1%	✓

Section	Description	Observed			Modelled			Validation			16:30 - 17:30			17:30 - 18:30			18:30 - 19:30								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates
15	M69 NB Lychgate Ln	2180m	68	79	80	80	80	67	79	80	12	18%	x	68	79	80	12	18%	x	68	79	80	12	18%	x
16	M69 NB End	633m	19	22	22	22	22	21	22	22	3	15%	✓	19	22	22	3	15%	✓	19	22	22	3	15%	✓
TOTAL	M69 NB Lychgate Ln	2813m	87	101	102	102	102	87	101	102	15	17%	x	87	101	102	15	17%	x	87	101	102	15	17%	x

Section	Description	Observed			Modelled			Validation			16:30 - 17:30			17:30 - 18:30			18:30 - 19:30								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates
17	M69 SB Start	603m	19	22	22	22	22	19	22	22	3	15%	✓	19	22	22	3	15%	✓	19	22	22	3	15%	✓
18	M69 SB Offlap	2197m	68	75	75	75	75	68	75	75	8	12%	✓	68	75	75	8	12%	✓	68	75	75	8	12%	✓
TOTAL	M69 SB Lychgate Ln	2801m	87	97	97	97	97	87	97	97	11	12%	✓	87	97	97	11	12%	✓	87	97	97	11	12%	✓

Section	Description	Observed			Modelled			Validation			16:30 - 17:30			17:30 - 18:30			18:30 - 19:30								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates
13	NBOnslp	472m	19	22	22	22	22	19	21	22	3	15%	✓	19	21	22	3	14%	✓	19	21	22	3	14%	✓
9	RAB Crc SB M69 Offlap to B4699	16m	1	1	1	1	1	1	1	1	0	-18%	x	1	1	1	0	-20%	x	1	1	1	0	-20%	x
10	RAB Crc SB	111m	8	7	7	7	7	7	7	7	0	0%	✓	7	7	7	0	0%	✓	7	7	7	0	0%	✓
11	RAB Crc WB	218m	14	11	12	12	12	13	11	12	-2	-17%	x	14	11	12	-2	-16%	x	14	11	12	-2	-16%	x
3	RAB 4699 EB Sapote Rd	234m	24	25	25	25	25	24	25	25	2	5%	✓	24	25	25	2	5%	✓	24	25	25	2	5%	✓
TOTAL	NBOnslp	1112m	66	66	67	67	66	66	66	67	2	2%	✓	66	66	67	2	2%	✓	66	66	67	2	2%	✓

Section	Description	Observed			Modelled			Validation			16:30 - 17:30			17:30 - 18:30			18:30 - 19:30								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates
13	NBOnslp	472m	19	22	22	22	22	19	21	22	3	15%	✓	19	21	22	3	15%	✓	19	21	22	3	15%	✓
9	RAB Crc SB M69 Offlap to B4699	16m	1	1	1	1	1	1	1	1	0	-18%	x	1	1	1	0	-20%	x	1	1	1	0	-20%	x
10	RAB Crc WB	111m	8	7	7	7	7	7	7	7	0	0%	✓	7	7	7	0	0%	✓	7	7	7	0	0%	✓
11	RAB Crc WB	218m	14	11	12	12	12	13	11	12	-2	-17%	x	14	11	12	-2	-16%	x	14	11	12	-2	-16%	x
3	RAB 4699 EB Sapote Rd	234m	24	25	25	25	25	24	25	25	2	5%	✓	24	25	25	2	5%	✓	24	25	25	2	5%	✓
TOTAL	NBOnslp	1177m	83	87	88	88	88	80	86	87	7	9%	✓	80	86	87	7	9%	✓	80	86	87	7	9%	✓

Section	Description	Observed			Modelled			Validation			16:30 - 17:30			17:30 - 18:30			18:30 - 19:30									
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	
2	B4699 WB Sapote Rd - Smyth Ln	476m	25	24	25	25	25	24	24	24	25	0	1%	✓	24	24	25	0	1%	✓	24	24	25	0	1%	✓
7	RAB Crc B4699 WB	15m	1	1	1	1	1	1	1	1	1	0	31%	✓	1	1	1	0	32%	✓	1	1	1	0	32%	✓
14	SB Offlap	430m	23	22	22	22	22	23	22	22	-1	-4%	✓	23	22	22	-1	-5%	✓	23	22	22	-1	-4%	✓	
TOTAL	B4699 WB Sapote Rd - Smyth Ln	921m	48	47	47	47	48	48	47	47	-1	-1%	✓	48	47	47	-1	-1%	✓	48	47	47	-1	-1%	✓	

Section	Description	Observed			Modelled			Validation			16:30 - 17:30			17:30 - 18:30			18:30 - 19:30								
		From	To	Dist.	Min.	Avg.	Max.	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates	Min.	Avg.	Max.	Actual Diff. % Diff.	Within 1 min.	Validates
6	B4699 WB Sapote Rd - Woodside C	530m	29	29	29	29	29	29	29	29	0	0%	✓	28	29	29	0	0%	✓	28	29	29	0	0%	✓
11	RAB Crc WB	218m	5	8	8	8	8	4	8	8	4	78%	✓	4	8	8	4	91%	✓	4	8	8	4	85%	✓
7	RAB Crc B4699 WB	15m	1	1	1	1	1	1	1	1	0	31%	✓	1	1	1	0	32%	✓	1	1	1	0	32%	✓
14	SB Offlap	430m	23	22	22	22	22	23	22	22	-1	-4%	✓	23	22	22	-1	-5%	✓	23	22	22	-1	-4%	✓
TOTAL	B4699 WB Sapote Rd - Woodside Garage	2059m	108	108	108	108	108	106	108	108	2	2%	✓	107	108	108	2	2%	✓	107	108	108	2	2%	✓

TRANSPORT & INFRASTRUCTURE PLANNING

Tritax Symmetry Ltd
Hinckley National Rail Freight Interchange
M69 Junction 1 & 2 VISSIM
Audit Response



TRANSPORT & INFRASTRUCTURE PLANNING

Tritax Symmetry Ltd

Hinckley National Rail Freight Interchange

M69 Junction 1 & 2 VISSIM

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TABLES

Table 1: Highways England Comments & BWB Response

Table 2: HE Spreadsheet Audit Comments

Table 3: LCC Audit Comments

Table 4: Further Audit Response

Table 5: AM Flow Calibration – 0730 – 0830 hrs

AM Peak (07:30-08:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 6: AM Flow Calibration – 0830 – 0930 hrs

AM Peak (08:30-09:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 7: AM Flow Calibration – 0730-0930 hrs

AM Peak (08:30-09:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 8: PM Flow Calibration - 1630-1730 hrs

PM Peak (16:30-17:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 9: PM Flow Calibration -1730-1830 hrs

PM Peak (17:30-18:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 10: PM Flow Calibration – 1630-1830 hrs

PM Peak (16:30-18:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 11: AM Journey Time Validation

Table 12: PM Journey Time Validation

Table 13: LCC M69 Junction 1 Comments

Table 14: LCC M69 Junction 2 Comments

Table 15: WCC Modelling Comments

1. INTRODUCTION

Instruction

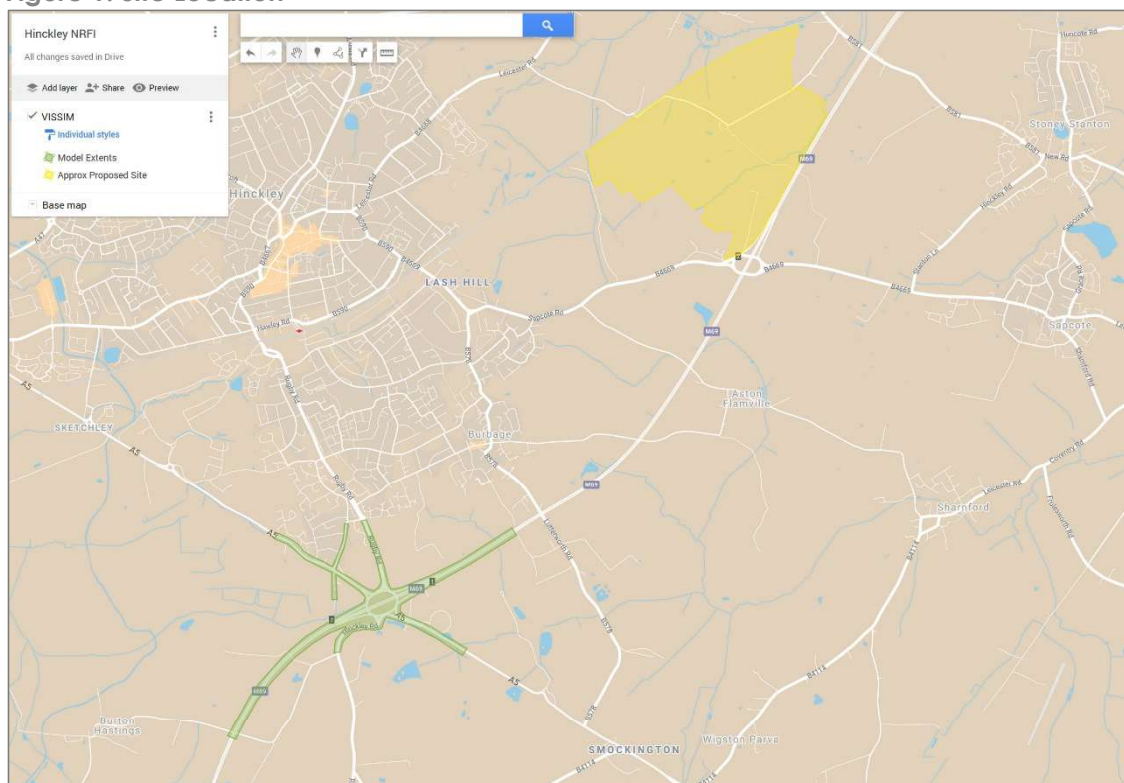
- 1.1 BWB Consulting has been commissioned as part a wider project scope by Tritax Symmetry Ltd to develop a series of highway models capable assessing any highway impacts resultant of the proposed Hinckley National Rail Freight Interchange (HNRFI) development. It is understood that the site will be developed serving a maximum of 850,000sqm of B8 warehousing/distribution uses, with access served directly onto M69 Junction 2.

The model purpose is to provide a robust platform on which the proposed development can be tested, allowing any impacts on the junction and surrounding highway network to be assessed.

Site Location

- 1.2 **Figure 1** below displays the indicative location of the proposed development , as well as the relative position of the highway model extents.

Figure 1: Site Location



Report Purpose

- 1.3 Due to the scale of the proposed development and the likely vehicular trips that it will generate, a comprehensive micro-simulation model of both the M69 Junction 1 and 2 gyratory has been developed using PTV Group's VISSIM software.

- 1.4 The following Local Model Validation Report (LMVR) summarises the methodology used to build and test the model, as well as the results obtained to determine the suitability of the model for use in proposed option testing.
- 1.5 Following the completion of the validation process, the model will be submitted for approval to Highways England (HE) and Leicestershire County Council (LCC) as the Local Highway Authority (LHA), for review, comment, and agreement. Once traffic outputs are available from the strategic LLITM (Leicester and Leicestershire Integrated Transport Model), the development proposals will be assessed.
- 1.6 This document provides commentary from Highways England, Leicestershire County Council & Warwickshire County Council after their review and comment. The comments and BWB's response are shown on the following pages.

2. REPORT STRUCTURE

The report is structured as follows:

- Section 3: Model review commentary from Highways England and BWB modelling response.
- Section 4: Model review commentary from Leicestershire County Council and BWB modelling response.
- Section 5: Model review commentary from Warwickshire County Council and BWB modelling response.
- Section 6: Observed Travel Time data interrogation

3. Model Review – Highways England

3.1 HE provided comments with regards to M69 Junction 1 modelling on 18/02/2021. BWB response has been provided in **Table 1** and **2**.

Table 1: Highways England Comments & BWB Response

Modelled Network	
Highways England Comment	BWB Modelling Response
Overlapping was observed on numerous occasions, where vehicles (both light and heavy) do not always give way from the B4109 south;	Priority Rules 31 & 32 amended to reduce this occurrence
Weaving issues have been identified at the M69 Southbound on-slip where most vehicles attempt a late merging manoeuvre. This may be caused by the coding of the circulatory (traffic travelling to the M69 southbound on slip comes around the roundabout in two lanes before merging) which differs from how the junction is marked on satellite imagery. This results in traffic waiting on the circulatory and then blocking traffic from exiting the B4109 south.	Connector 10045 Lane Change distance to be investigated. The lane merging is at the optimum location within the model - increasing the lane change distance causes issues to the circulating traffic whereas decreasing the distance makes more later merging and vehicle occurrences.
A proportion of traffic travelling from the A5 northwest to both the A5 southeast or B4109 south use the wrong lane (i.e. the offside lane) on approach to the junction and then weaves on the circulatory.	We will investigate this - there may be an opportunity to stop lane change on a number of the links namely: 15, 10109, 97 however we would want these to be as minimal as possible as we do not necessarily want to keep this arrangement in the future. Vehicles will inherently change lanes within the circulatory of a roundabout - by banning lane change we are forcing vehicles to stay in lanes where in reality they are allowed.
Vehicles travelling from the M69 northbound to the A5 northbound use both nearside lanes. Road markings on satellite imagery show that the nearside lane only should be used for this movement.	This is a valid manoeuvre - vehicles can make both movements from these lanes. See Figure 2 overleaf which outlines all available movements in the model but also when referring to the lane markings on site.
The two nearside lanes are marked for the B4109 but in the model it is only possible to make this movement in the middle lane.	This manoeuvre is open to any lane - there are look back 'Lane Change Distance' parameters which dictate when vehicles start to change lanes to be at the correct location within the model. See Figure 3 overleaf which provides details on which links and connectors each movement would use within the model.

Figure 2: M69 NB Offsip to A5 NB or Circulatory



Figure 3: M69 NB Offslip Connectorsw

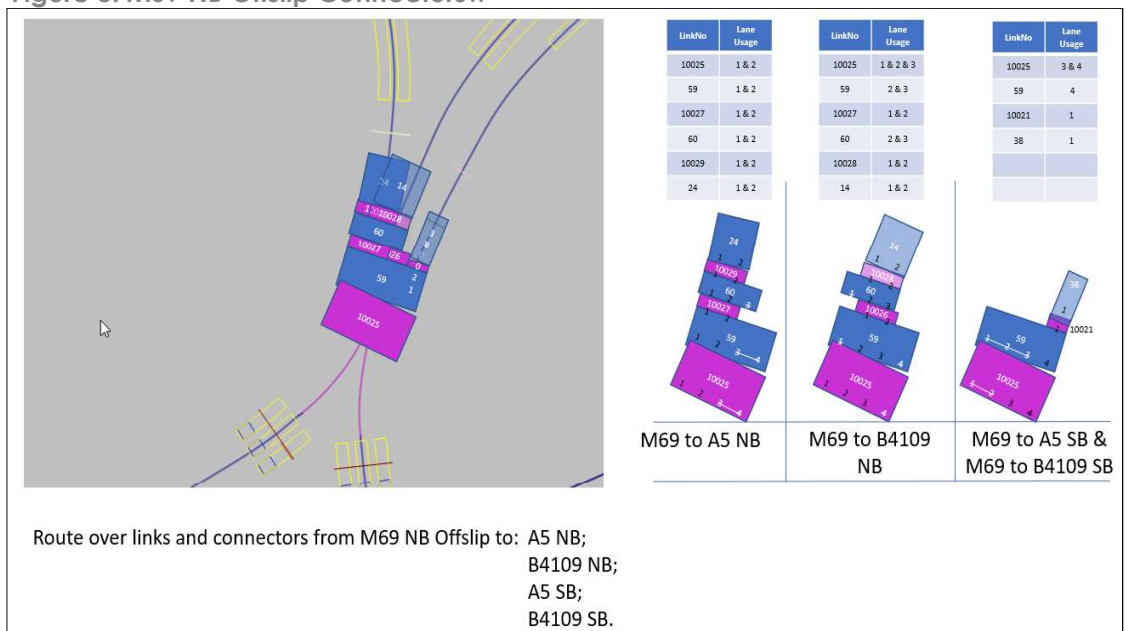


Table 2: HE Spreadsheet Audit Comments

Spreadsheet Audit Commentary	
Highways England Comment	BWB Modelling Response
In the 'SurveyChecks_JT_AM' the observed values are looking up the wrong time periods. For example, for the time period 0730-0745 the observed value in cell F7 reads 113 seconds. However, in the 'M69_TT_J1.xlsx' spreadsheet 113 seconds is for 0700-0715. It appears that the observed journey times in the CalVal spreadsheet are reading the data for 30 minutes before. This	Spreadsheet changed Cell F1 to 4 rather than 2 fixes this and refer to the correct travel time dataset. See below for further details related to travel times.

<p>also occurs in the PM. This should be corrected, and validation of the models checked against the updated values and an updated LMVR provided.</p>	
<p>The hourly journey times for which the final validation is undertaken is calculated by averaging the four average 15-minute periods. Averaging an average can result in skewed results for the whole hour, therefore it is recommended that the 15-minute journey times are weighted by the 15-minute flow to calculate an hourly average.</p>	<p>Amended calculations to weight the travel times as suggested. All TT still validated as previous. Option available to turn Weighted Average On / Off.</p>

- 3.2 It should be noted that no saturation flow data was made available. Model Specification. At each stopline within the model, there are Reduced Speed Areas coded to represent 'standard' traffic behaviour for vehicles crossing signal stop lines and thus standard saturation flows,

4. Model Review - Leicestershire County Council

Table 3: LCC Audit Comments

Modelled Network Junction 1	
Leicestershire County Council Comment	BWB Modelling Response
Route Convergence: it is unusual for a Dynamic Assignment model to not be run to convergence; admittedly there is limited route choice, but further clarity should be provided around the decision not to converge the model (noted that in LMVR section 3.14 there is a short section on this matter) and what impact would converging the model have on the validation?	See <i>Dynamic Assignment Clarification</i> section

Dynamic Assignment Clarification

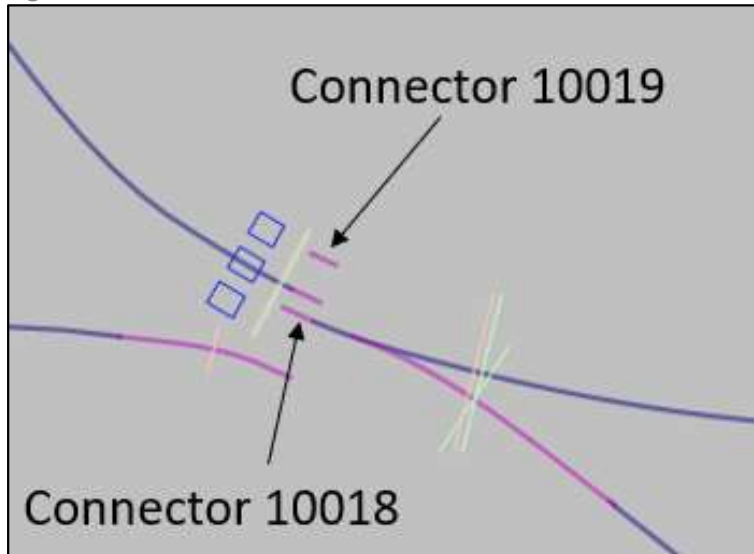
- 4.1 Further to this question, and after review of the model HE commented that: Leicestershire County Council provided a model response on 19/02/21. See **Table 4** below for details:

Table 4: Further Audit Response

HE Comment
Although it is not part of Highways England's previous comments, we note that LCC raised a query over model convergence not having been assessed. We would like to note that the model does have route choice as it stands. These route choices are inherently created as a result of connectors 10018 and 10019, which are open for all paths. Hence, creating multiple edges (for example, edges numbers 147, 148, 492 and 497) for the same movement. We therefore advise you review the open edges passing through these connectors or provide a convergence assessment, if applicable.

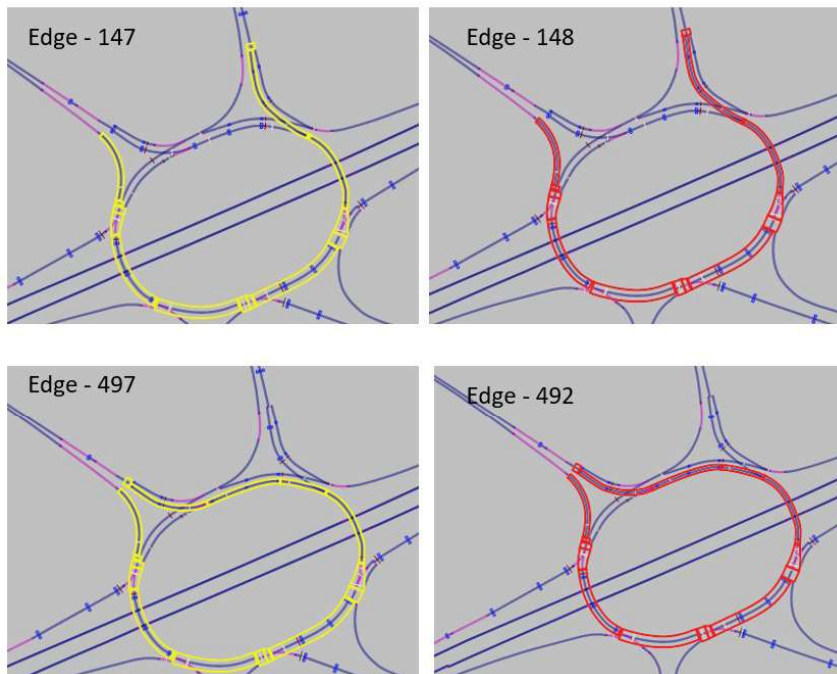
- 4.2 BWB acknowledge that in our response, it was stated that there was not any route choice for the modelled extent. This statement was made when looking at the network as a whole – there are no Origin / Destination pairs where there are two routes, and not looking at the specific network structure.
- 4.3 The statement made by LCC and HE are correct inasmuch that there is route choice within the model as routes travelling around the gyratory could use either connector 10018 and 10019.

Figure 4: Connector Structure



- 4.4 When a VISSIM model is built to use Dynamic Assignment, when the traffic is assigned, an abstract network is automatically created. This abstract graph refers to junctions as nodes and links between junctions as edges. In the previously submitted model, edges 147 and 148 are used for travel between Rugby Road SB to the A5 northbound and edges 492 and 497 from Rugby Road SB to Wolvey Road NB.

Figure 5: Dynamic Assignment Edges



- 4.5 When the routing assignment is calculated, the network is simulated repetitively with the vehicles choosing their paths through the network based on the best path through the network. If there are multiple routes, vehicles are assigned to the routes – in the case of the M69 J1 model there should be no route choice available.

Re-convergence requirement

Multiple Routes

- 4.6 In order to stop the route choice within the model, one of the edges for each OD pair must be closed. Edges 148 and 492 are required to be closed. (see Figure 5). These edges use connector 10019 which is located on lane 3 of the gyratory and not the lanes that should be used for vehicles leaving the gyratory to the northbound A5 exit. With this edge open, vehicles would be in the incorrect lane to be able to safely leave the gyratory at the A5 northbound exit.
- 4.7 Whilst edges 148 and 192 have been closed, it should be noted that other routes are still able to use this connector – specifically vehicles entering the gyratory from the B4109 Hinkley Road and making the movement to any exit other than the M69 WB or the A5 NB exits.
- 4.8 Unfortunately, closing an edge after compiling a Dynamic Assignment model does not automatically move modelled vehicles from the closed edge onto alternative edges. A full re-convergence of the model is therefore required where all potential routes are analysed. In doing a full re-convergence exercise, all available edges are assessed for each path between OD pairs and the vehicles are assigned accordingly. However, as there is now no route choice between OD pairs (due to the closure of edges 148 and 492), there will be one route between each OD pair and technically convergence calculations would be unnecessary.
- 4.9 For completeness and to satisfy the auditor query, a convergence assessment has been carried out for both the AM and PM base model. As there is a potential change in the traffic movement around the gyratory, the model validation will also be checked however there is envisaged to be no change to the status of the model.

Convergence Criteria

- 4.10 The Transport for London Modelling Guidelines suggest that the following convergence criteria are fulfilled:
- **95% of all path traffic volumes change by less than 5% for at least four consecutive iterations; and**
 - **95% of travel times on all paths change by less than 20% for at least four consecutive iterations.**
- 4.11 And the DMRB TAG criteria adds:
- **The percentage change in user costs or time spent within the network (V) should be less than 1% for four consecutive iterations.**
- 4.12 Using the two criteria, BWB have analysed the convergence of the base models using Path Travel Times and Volumes for 30 iterations. The results are shown below.

4.13 As there is no route choice, the traffic volumes will have minimal difference between model iterations and the overall travel times between the iterations will also show minimal differences.

Figure 6: AM Model Convergence

AM					
Run Number	Paths			Network Performance	
	Travel Times (0-20%)		Vol. Difference (0-5)	Total Travel Time (s)	Diff from prev. run
	Check 1 - ShrConvPathTT	Check 2 - Paths List - Converged = Yes			
1	100%	100%	98%	1218858.5	-
2	100%	100%	100%	1219514.5	0.1%
3	100%	100%	100%	1224328.2	0.4%
4	100%	100%	100%	1216447.4	-0.6%
5	98%	100%	100%	1220028.3	0.3%
6	100%	100%	100%	1218548.7	-0.1%
7	100%	100%	100%	1218877.6	0.0%
8	100%	100%	100%	1219728.6	0.1%
9	100%	100%	100%	1218737.4	-0.1%
10	100%	100%	100%	1219978	0.1%
11	100%	100%	100%	1218542.1	-0.1%
12	100%	100%	100%	1222335.8	0.3%
13	100%	100%	100%	1222748.8	0.0%
14	100%	100%	100%	1220483.4	-0.2%
15	100%	100%	100%	1223055.6	0.2%
16	100%	100%	100%	1217468.6	-0.5%
17	100%	100%	100%	1217206.3	0.0%
18	100%	100%	100%	1216967.3	0.0%
19	100%	100%	100%	1217347.3	0.0%


 Most suitable run - BEW and WEG used for results reporting

Figure 7: PM Model Convergence

PM					
Run Number	Paths			Network Performance	
	Travel Times (0-20%)		Vol. Difference (0-5)	Total Travel Time (s)	Diff from prev. run
	Check 1 - ShrConvPathTT	Check 2 - Paths List - Converged = Yes			
1	100%	100%	100%	1334536	-
2	100%	100%	98%	1347807.1	1.0%
3	100%	100%	98%	1338237.6	-0.7%
4	100%	100%	98%	1344255.1	0.4%
5	100%	100%	98%	1342350.1	-0.1%
6	100%	100%	98%	1343548	0.1%
7	100%	100%	98%	1341142	-0.2%
8	100%	100%	98%	1340436.6	-0.1%
9	100%	100%	98%	1343589	0.2%
10	100%	100%	98%	1361550.3	1.3%
11	100%	100%	98%	1341739.9	-1.5%
12	100%	100%	98%	1338824.6	-0.2%
13	100%	100%	98%	1335770	-0.2%
14	100%	100%	98%	1339773.2	0.3%
15	100%	100%	98%	1344630.5	0.4%
16	100%	100%	98%	1342177	-0.2%
17	100%	100%	98%	1335539.2	-0.5%
18	100%	100%	98%	1354537.3	1.4%
19	100%	100%	98%	1341573.3	-1.0%
20	100%	100%	98%	1343467.1	0.1%
21	100%	100%	98%	1325857.1	-1.3%
22	100%	100%	98%	1354584.3	2.2%
23	100%	100%	98%	1349711.9	-0.4%
24	100%	100%	98%	1344865.2	-0.4%
25	100%	100%	98%	1337004.1	-0.6%
26	100%	100%	98%	1344690.3	0.6%
27	100%	100%	98%	1339054.9	-0.4%
28	100%	100%	98%	1346476.1	0.6%
29	100%	100%	98%	1344987.8	-0.1%
30	100%	100%	98%	1343135	-0.1%

Most suitable run - BEW and WEG used for results reporting

Calibration & Validation of Model

4.14 The following tables provide a summary of the status of the model after the re-convergence exercise.

Flow Calibration

Table 5: AM Flow Calibration – 0730 – 0830 hrs

AM Peak (07:30-08:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 6: AM Flow Calibration – 0830 – 0930 hrs

AM Peak (08:30-09:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 7: AM Flow Calibration – 0730-0930 hrs

AM Peak (08:30-09:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 8: PM Flow Calibration - 1630-1730 hrs

PM Peak (16:30-17:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 9: PM Flow Calibration - 1730-1830 hrs

PM Peak (17:30-18:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Table 10: PM Flow Calibration – 1630-1830 hrs

PM Peak (16:30-18:30) Summary - ALL	
Total number of counts considered	42
VISSIM model counts with GEH <3	42
% of VISSIM counts with GEH <3	100.00%
VISSIM model counts with GEH <5	42
% of VISSIM counts with GEH <5	100.00%
VISSIM model counts with GEH <10	42
% of VISSIM counts with GEH <10	100.00%
VISSIM model counts meeting WebTAG Unit 3.1 criteria	42
% of VISSIM counts meeting WebTAG Unit 3.1 flow criteria	100.00%

Vehicle Journey Time Validation

- 4.15 As before, the journey time validation has been carried out using TomTom data.
- 4.16 A total of 12 journey time routes have been prepared for the purpose of model validation. (Four primary routes and eight secondary routes).

Figure 8: Journey Time Routes - Primary



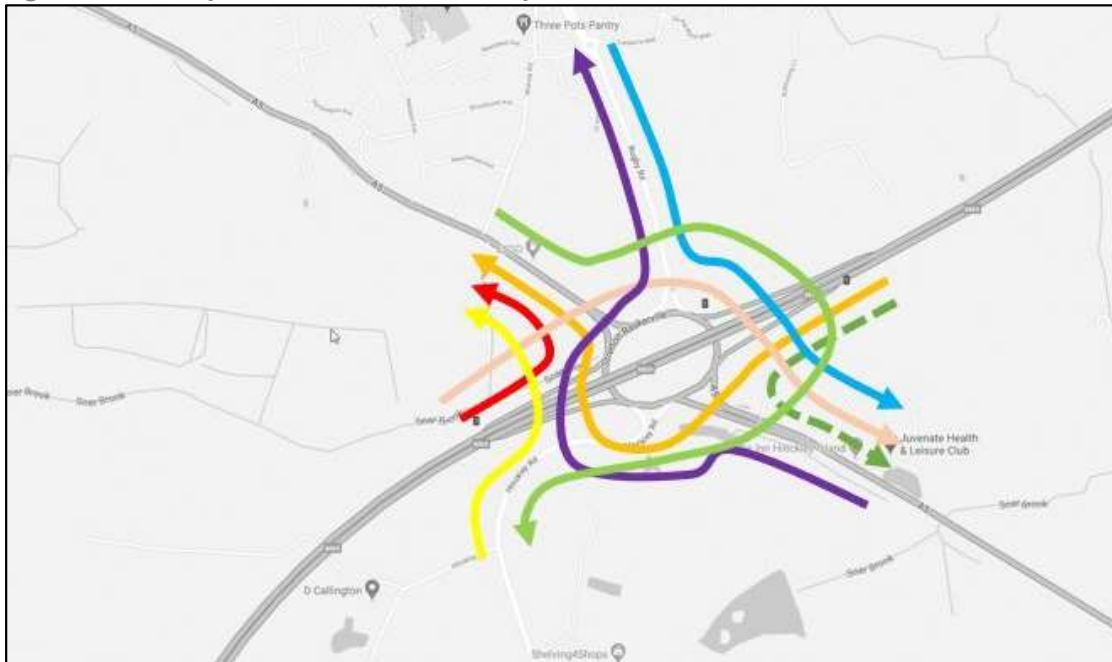
Table 11: AM Journey Time Validation

Whole Routes	AM Peak		
	07:30-08:30	08:30-09:30	07:30-09:30
Criteria			
85% of measures within 15%	83%	92%	100%
85% of measures within 60 seconds	100%	100%	100%

Table 12: PM Journey Time Validation

Whole Routes	PM Peak		
	16:30-17:30	17:30-18:30	16:30-18:30
Criteria			
85% of measures within 15%	100%	92%	100%
85% of measures within 60 seconds	100%	100%	100%

Figure 9: Journey Time Route - Secondary



- 4.17 In accordance with TAG Unit 3.1, which recommends that the difference between observed and modelled journey times should be within 15% (or 1 minute if higher) for at least 85% of the routes evaluations, it can be seen that all routes meet one of both criteria in the AM and PK peak models.
- 4.18 In the AM peak, the 0830-0930 hrs and 0730-0930 hrs time periods both meet the TAG criteria with over 85% of the routes being within 15% and 60s. In the 0730-0830hrs time periods, there are two routes which fall outside of the 15% difference. (one having 16% difference) and 12/12 routes are within the 60s. Given how closed the non-validating route is to the 15% difference, the model is still considered representative of on-street conditions.
- 4.19 In the PM peak, all time periods are within 15% and 60s. Therefore, the PM model is considered representative of on-street conditions.
- 4.20 LCC provided further comments on 23/02/21, BWB modelling responses have been provided in **Table 13** and **14**.

Table 13: LCC M69 Junction 1 Comments

Modelled Network Junction 1	
Leicestershire County Council Comment	BWB Modelling Response
Reduced Speed Areas; suggest adding a RSA on A5 south east bound exit of M69 junction as this is a sharp turning.	Noted - added Reduced Speed Area No. 60 is on the A5 SB link
A5 section between Wolvey Road and M69 Roundabout should be National Speed Limit (both directions)?	Added Desired Speed Decisions (DSD) 45,47, 50,51, 53, 54,55 A5 NBSB NSL & 40mph Wolvey Rd & B4109 NB @ 50mph and SB @ NSL

A5 section South east of M69 junction should be National Speed Limit (both directions)?	Noted - added
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Table 14: LCC M69 Junction 2 Comments

Modelled Network Junction 2	
Leicestershire County Council Comment	BWB Modelling Response
The vehicle behaviour around Link 50 should be improved. Currently vehicles observed to slow down to around 25kph on the approach to this link which impacts on the circulatory flow of the roundabout.	It is acknowledged that there is hesitancy with some vehicles as they travel around the circulatory travelling through Link 50. The link structure in this section was updated in an attempt to better reflect the lane markings and layout on site. In VISSIM, the use of the 0.1m lane technique has been used to model the flare from Lane 2 on the circulatory to 3 further downstream (highlighted in yellow below).



- 4.21 There is an argument that in reality, the flare should be from Lane 2 to Lane 2, rather than Lane 2 to Lane 3. However, in VISSIM, there is a limitation that the 0.1m lane cannot be included in the middle of the link without needing separate connectors. This approach was considered, but it was felt that this would fix the lane use too much to specific lanes, where the existing layout shows more lane choice.
- 4.22 It should also be noted that the flow calibration on this approach meets TAG guidance and the journey times that include this section are all within TAG criteria, indicating that the operation is not significant enough to affect the comparisons with observed data.
- 4.23 Finally, with reference to the observations by Leicestershire County Council for the Junction 2 model – we have further investigated this and have noticed that the observed TRADS data reports a different traffic flow on the slips when compared to the observed survey dataset of which it is more prevalent in the PM peak.
- 4.24 The observed MCC data for the J2 Onslip is 1002 vehs whereas the TRADS is 1211 vehs. The modelled is reporting as 1071 vehs.

Validation compared to MCC data:

Junction	Junction/ Movement		Vehicle Flow		Difference		GEH Criteria Met			Flow Criteria Met			
	Approach	Exit/movement	Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW	<700	700 - 2700	>2700
M69 North		B4669 East	346	365	19	5%	N	1.01	✓	✓			
		B4669 West	656	706	50	8%	N	1.92	✓	✓			

Note – the slip flows in the MCC data are calculated using both the 364 and 706 values.

Validation compared to the TRADS dataset:

Link	Junction/ Movement	Vehicle Flow		Difference		GEH Criteria Met			
		Observed	Modelled	Actual	%	Critical	GEH	Pass	FLOW
M69 J2 Off-Slip	Southbound	1211	1071	-140	-12%	Y	4.14	✗	✓

- 4.25 We included the analysis within the LMVR for completeness as the flows are within 12% of the observed TRADS and are within TAG guidance. However if we look at the MCC validation the difference is 5% and a GEH if 1.01 for the east movement and 8% difference and a GEH of 1.92 for the west so validated very well.

5. Model Review - Warwickshire County Council

5.1 Warwickshire County Council provided modelling review comments on 23/02/21, BWB response has been provided in **Table 15** below..

Table 15: WCC Modelling Comments

Modelled Network Junction 1	
Warwickshire County Council Comment	BWB Modelling Response
<p>LMVR for M69 jct 1 and would just like clarification as to why the for the journey time assessment the B4109 Hinckley Rd (from Wolvey) is only considered to/from A5 Southbound (towards Dodwells)?</p> <p>We appreciate that when the revisions for the comments that HE and LCC have made that the results will be updated, however it would assist us to understand what the 'sections' are on the network and which 2 journey time routes are outside of the acceptability criteria</p>	<p>Noted. The travel time data provided by Tom Tom is for individual link segments. When compiling a route, each of the link segments are combined to create a full route. There are some sections – predominantly on the gyratory – where multiple routes traverse the same link segments. So that each of the individual segments are included in the model validation, we have selected routes that cover all segments at least once.</p> <p>We have expanded the text within the LMRV to read:</p> <p><i>The data is provided in small link sections, so these were combined into more reasonable lengths from junction to junction in the network, which assisted the calibration of the model. For the purpose of providing journey time validation, multiple sections have been combined into longer journey routes, covering all major movements at key locations.</i></p> <p><i>Within the gyratory, at least three full routes have been selected for each section.</i></p> <p><i>Again, we have expanded the LMRV to detail any Travel Time segments that do not validate and provide the differences.</i></p>

6. Model Review - Observed Travel Time data interrogation

- 6.1 When looking at the travel time segments within the model, it was seen that some of the observed data on one section did not make sense.
- 6.2 The section of interest was found on A5 NB route and was segment number 10. This segment contains a number of individual Tom Tom sections namely – id 492, 434, 93, 250, 270 and 560, as shown in the following image.

Figure 10: Journey Time Segment



- 6.3 The travel times shown from the TOM TOM dataset show that in the 0745-0800 period the travel time section took 192s to traverse where in the previous 3 15minutes periods the travel time was recorded as 76s, 73s and 67s respectively. Then for the remaining periods, the travel time drops to 94s and less. Clearly there was an incident or erroneous data capture.

Section	AM Journey Time (s)											
	0700-0715	0715-0730	0730-0745	0745-0800	0800-0815	0815-0830	0830-0845	0845-0900	0900-0915	0915-0930	0930-0945	0945-1000
10	76	73	67	192	94	87	54	58	55	53	60	52

- 6.4 The TomTOM data set contained the single day Wednesday 10th April 2019 data but also as an average for the month of May 2019. This dataset has been used to replace the erroneous data period for the whole segment for consistency. The travel time measurements now report the following:

Section	AM Journey Time (s)											
	0700-0715	0715-0730	0730-0745	0745-0800	0800-0815	0815-0830	0830-0845	0845-0900	0900-0915	0915-0930	0930-0945	0945-1000
10	81	74	80	69	88	69	58	69	63	56	60	52

- 6.5 So that both peaks are using the data dataset, the average monthly data has been used for the PM peak also.

7. SUMMARY AND CONCLUSIONS

SUMMARY

- 7.1 This LMVR Audit Response (and the associated revised LMVR) document(s) have been provided to show the development of the base model and further demonstrates that it is an acceptable representation of the highway network within the study area and is fit for the purpose of developing traffic forecasts to assess the impact of development proposal scheme on the M69 Junction 1 gyratory.
- 7.2 BWB have taken all comments onboard and where necessary updated the revised the model to reflect these changes.
- 7.3 The purpose of model calibration is to ensure that the model assignments are appropriate. The main emphasis of the calibration is to ensure that the model accurately reflects existing conditions during the modelling period with regard to:
- Traffic patterns;
 - Key junctions; and
 - Traffic volumes and routing.
- 7.4 In regard to the traffic turning and flow counts at the surveyed sites the model exceeds the 85% criteria set by TAG Unit 3.1.
- 7.5 The model has been validated to observed journey times within the extents of the network. The data has been provided as 15-minute intervals and an average one-hour journey time for each of the peak network periods.
- 7.6 The journey time comparisons show consistency between the modelled and observed journey time profiles across the majority of the journey time routes. In the AM and PM peaks, there are a number of routes which fall outside of the 15% range (0730-08hrs and 1730-1830hrs). However, all of the routes are **well** within 60s and as such, the journey times are considered representative.
- 7.7 Given that the traffic flows and journey times compare well with on-site conditions, it should be considered a successful calibration and validation exercise.

CONCLUSION

- 7.8 It is understood that as these conditions are met and are of a sufficient quality to represent real world conditions the M69 Junction 1 and Junction 2 gyratory VISSIM models are considered robust and acceptable for testing of the proposed development.



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